STEADY-STATE AND DYNAMIC BEHAVIOUR
OF COMBINED AND SEPARATE SLUDGE
.CARBON REMOVAL-NITRIFICATION
SYSTEMS

## STEADY-STATE AND DYNAMIC BEHAVIOUR OF COMBINED AND SEPARATE SLUDGE CARBON REMOVAL-NITRIFICATION SYSTEMS

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## ABSTRACT

This dissertation examines the degree of nitrification which can be accomplished in combined and separate activated sludge systems over a temperature range of  $5^{\circ}$  to  $25^{\circ}$ C and a system solids residence time range of 4 to 10 days under both steady and non-steady operating conditions.

Treating municipal sewage under steady flow conditions, it was found that the rate of nitrification was independent of the concentration of filterable TKN or ammonia. Temperature and solids retention time significantly affected filterable TKN removal. The degree of nitrification obtained in both combined and separate sludge systems was comparable.

The parallel pilot plant systems were subjected to a number of non-steady influent conditions. The responses to a pulse change in influent pH and a step-down in temperature indicated that the separate sludge system had a greater capacity to withstand such conditions. Transfer function models, together with time series models, were developed to describe the dynamic responses of the nitrifying systems to changes in influent flow, and organic carbon and inorganic nitrogen concentration. The observed and model results indicated greater effluent filterable TKN variation can be expected from nitrifying systems operated under variable flow and concentration inputs than for variable concentration inputs alone.

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### INTRODUCTION

The original approach to waste treatment was to provide processes for the removal of settleable solids, the reduction of biochemical oxygen demand (BOD), and the elimination of bacterial contaminants. In recent years, eutrophication problems have focused attention on the requirement for nutrient control; phosphorus or nitrogen are considered to be the nutrients limiting algal production. As phosphorus was identified as the major pollutant, an intensive research program conducted over the past decade, has resulted in the development and full scale evaluation of phosphorus removal processes which are practical and economical.

Recently, research has led to the development of nitrogen removal technology. Deficiencies in process design for nitrogen removal still exist as clearly pointed out in this quotation from the Report of the WPCF Research Committee (1975);

"There have been many studies on the removal of nitrogen from wastewaters, but clear-cut design parameters that permit the application of treatment processes without difficulty have yet to be established. Pilot or large scale studies are now required to demonstrate the efficiency and reliability of available processes and attendant operational problems and their remedy."

Nitrogen control can be divided into two categories. The first involves processes which convert organic and ammonia nitrogen into nitrate nitrogen. The second category involves processes resulting in the complete removal of nitrogen from the wastewater.

The first category normally is termed nitrification and is effective in eliminating many problems associated with organic and ammonia nitrogen. The biochemical oxygen demand (BOD) of municipal wastes will be substantially influenced by the presence of ammonia. The conversion of ammonia to nitrate requires 4.6 parts of oxygen for each part of ammonia nitrogen. Therefore, ammonia in the effluent at concentrations of 20 mg/l as N would give a theoretical nitrogenous oxygen demand of 92 mg/l.

Additional reasons for ammonia nitrogen removal include:

- ` 1) NH<sub>3</sub> at low concentrations is toxic to fish,
  - 2) NH<sub>3</sub> is corrosive to copper fittings,

- 3) NH<sub>3</sub> increases Cl<sub>2</sub> breakpoint requirements and contact time for adequate disinfection, and
- 4) eutrophication problems are associated with high nitrogen effluents.

Nitrogenous compounds enter the aquatic environment from natural and man-induced sources. Natural sources include dustfall, precipitation, nonurban runoff, and biological fixation. The quantities from natural sources can be increased by man's activity. For example, the combustion of fossil fuels can increase rainfall concentrations of nitrogen substantially. Nitrogen sources which can be directly attributed to man's activity include:

municipal wastewater,

100

- 2) industrial wastewater,
- 3) runoff from urban areas,
- 4) runoff from livestock feedlots, and
- 5) drainage from agricultural lands.

The significance of each source will vary depending on the population density, the degree of industrial development, and the farming practices in each particular area. An estimate of the nitrogen quantities, from various sources discharged in the San Francisco Bay Basin, California, is given in Tablé 1. Owing to the high degree of industrialization and large population density, municipal and industrial contributions are most significant.

Nitrogen control can be accomplished by either biological or physical-chemical means. A review of the advantages and disadvantages of a number of control alternatives (Sutton, Murphy, and Dawson, 1974) indicates that biological nitrification-denitrification may be the preferred approach. The popularity of this alternative is evident from the recent IAWPR conference proceedings (1975) entitled "Conference on Nitrogen as a Water Pollutant".

Biological nitrogen removal is essentially a two-step process, nitrification followed by denitrification. In the nitrification step, under aerobic conditions, autotrophic nitrifying organisms oxidize ammonia to nitrate. In the denitrification step, nitrate is reduced to molecular nitrogen by heterotrophic organisms in the absence of molecular oxygen.

TABLE 1 NITROGEN LOADINGS FOR THE SAN FRANCISCO BAY BASIN (Brown and Caldwell, 1975)

Nitrogen Source	Nitrogen Discharge 1000 kg/yr	Percent of total
Municipal wastewater, before treatment	26,000	49
Industrial wastewater, before treatment	16,000	30 ·.
Vessel wastes, before treatment	60	0.1
Dustfall directly on Bay	<b>59</b> 0	1.1
Rainfall directly on Bay	390	9.8
Urban runoff	1,400	2.7
Non-urban runoff	1,900	3.6
Nitrogen applied to irrigated agricultural land	900	1.7
Nitrogen from dairies and feedlots	6,000	11
Total	53,000	100

. 1

Nitrification occurs in the activated sludge process when conditions are suitable for the retention and accumulation of nitrifying bacteria. The solids retention time (SRT), or sludge age, is a measure of the average retention time of the bacterial cells in the system. Successful nitrification depends on adherence to a sludge wasting program which results in an SRT adequate to retain and prevent the wash out of the slower growing nitrifying bacteria. Two basic process schemes available can be designated as:

- 1) a combined sludge system, and
- 2) a separate sludge system.

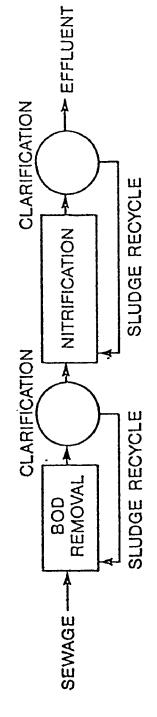
The combined sludge system may be a single or multi-stage system, while the separate sludge system is normally a two-stage system (Figure 1). In the combined sludge process, simultaneous carbon removal and nitrification are carried out. Provided that the rate of growth of nitrifying bacteria is sufficiently rapid to compensate for the organisms lost through sludge wasting, nitrification can be maintained. Consequently, nitrification depends upon the relationship of the growth rate of the nitrifying bacteria to the net solids production rate for the process. In the separate sludge system, carbon removal is carried out by heterotrophic microorganisms, separate and distinct from the subsequent nitrification step carried out by autotrophs. Thus separate sludge wasting procedures for each removal step can be incorporated.

There are advantages and disadvantages to both the combined and separate sludge systems. A separate system could be expected to offer more stability, less temperature sensitivity, and some buffering capacity to compounds toxic or inhibitory to nitrification. These advantages must be balanced against the possible overall increases in solids production and capital cost.

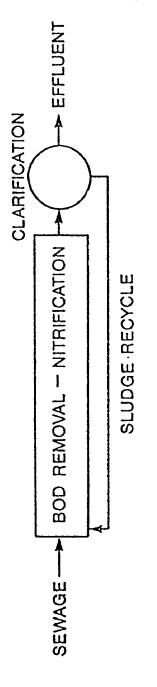
Parameter values derived from time averaged pseudo "steady-state" (constant flow with normal diurnal variations in organic and inorganic concentrations) data allow prediction of the average effluent concentration of filterable TKN (Sutton, Murphy, Jank, and Monaghan, 1975). This form of analysis may not be sensitive enough to differentiate between the alternative carbon removal-nitrification systems operating under "non-steady" conditions.



## TWO-STAGE SEPARATE SLUDGE



# SINGLE-STAGE COMBINED SLUDGE



# TWO-STAGE COMBINED SLUDGE

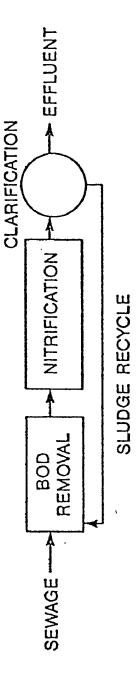


FIG. 1 SEPARATE AND COMBINED SLUDGE CARBON REMOVAL - NITRIFICATION SYSTEMS

Nitrification systems, like all activated sludge processes, are dynamic systems subject to time varying loadings and exhibiting a response to their environment which varies with time. Pseudo "steady-state" parameter values are not capable of describing the temporal relationships between inputs and outputs. The stability of the nitrification alternatives can be obtained only by using techniques which elucidate the responses to non-steady operation.

Significant changes in raw wastewater pH are encountered periodically at municipal treatment plants receiving quantities of industrial wastes. Variations in temperature may be anticipated in treatment facilities in continental climates. The reported sensitivity of nitrification to temperature and pH (Wild, Sawyer, and McMahon, 1971) emphasizes the importance of defining the tolerance of such systems to abrupt changes in these factors.

This study has examined, at pilot-scale, the carbon removal and nitrification efficiencies obtained from three differing process configurations - a "single" and "two-stage" combined sludge systems (denoted SSC and TSC respectively) and a two-stage separate sludge system (TSS). The alternatives (Figure 1) were compared as to their effectiveness, in terms of nitrification, under a range of operating temperatures and solids retention times, at pseudo "steady-state" conditions.

Experimental observation, mathematical model building and simulation, and time series analysis are techniques by which the behaviour of dynamic processes can be assessed. In this study, these methods were utilized to compare the alternative carbon removal-nitrification schemes and provide a better understanding of factors affecting nitrification under non-steady conditions.

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## **BACKGROUND**

## Principles of Biological Nitrification

## Microbiology and biochemistry

Biological oxidation of inorganic nitrogen is almost entirely carried out in the aquatic environment by bacteria belonging to the family Nitrobacteraceae. The major well defined autotrophic bacteria which are known to oxidize ammonia to nitrite are the genera Nitrosomonas and Nitrosococcus. The genera Nitrobacter and Nitrocystis have been shown to oxidize nitrite to nitrate. The ability to oxidize inorganic nitrogen is not restricted to autotrophic bacteria. Verstraete and Alexander (1973) have demonstrated that heterotrophic nitrification occurs in soils, in sewage treatment plants, and in river and lake waters. Although a large number of heterotrophs have been identified as nitrifiers (Painter, 1975), heterotrophic nitrification appears to be of much less importance than autotrophic nitrification.

The autotrophic nitrifying organisms derive all their energy requirements for growth and cellular metabolism from the free energy released by the oxidation of an inorganic nitrogen substrate. The source of carbon used for growth is obtained from carbon dioxide or bicarbonate. The bacterial oxidation reactions of ammonia to nitrite and nitrite to nitrate can be represented by the following equations:

$$NH_4^+ + 1.5 O_2 + 2 H^+ + H_2O + NO_2^-$$
 .....(1)

$$NO_2^- + 0.5 O_2^- + NO_3^-$$
 .....(2)

In the oxidation of ammonia to nitrite, nitrogen undergoes an oxidation state change from -3 to +3. This suggests that the reaction takes place in three steps since all known biochemical reaction mechanisms transfer one pair of electrons at a time. Kluyver and Donker (1926) postulated a three step sequence according to:

$$NH_4^+ + NH_2OH \rightarrow H_2N_2O_2 \rightarrow NO_2^-$$
 .....(3)

The intermediate, hydroxylamine (NH $_2$ OH), was reported first by Hofman and Lees (1953) and has since been demonstrated as an intermediate by several other researchers. The formation of the second oxidation product, hyponitrite ( $H_2N_2O_2$ ) has not yet been verified. In the oxidation of nitrite to nitrate, the nitrogen atom changes its oxidation state from +3 to +5 suggesting a one step reaction mechanism. No intermediates have been found in this enzymatic oxidation. The difference in free energy released in the oxidation reactions, 65.2 to 84.0 kcal/mole for ammonia oxidation and 17.5 to 24.0 kcal/mole for nitrite oxidation (Gibbs and Schiff, 1960) indicates that nitrite oxidation proceeds through a much simpler mechanism. The chemical energy released is stored or used in the form adenosine triphosphate (ATP). The generation of reducing compounds enables the nitrifiers to reduce carbon dioxide to the oxidation level of cellular components thereby removing the need for an organic carbon substrate as a carbon source.

## Reaction kinetics

The growth rate of a bacterial culture is a function of the concentration of some limiting substrate. For many biological systems the Monod kinetic model has been found to represent this growth rate and limiting substrate relationship:

$$u = \frac{u^* S}{K_S + S} , \qquad \dots (4)$$

where  $u = growth\ rate\ (T^{-1})$ ,  $u^*= maximum\ growth\ rate\ coefficient\ (T^{-1})$ ,  $S = substrate\ concentration\ (M/L^3)$ , and  $K_s = substrate\ concentration\ at\ one\ half\ maximum\ growth\ rate\ (M/L^3)$ .

Downing, Painter, and Knowles (1964), Poduska and Andrews (1975), and Knowles, Downing, and Barrett (1965) used the Monod kinetic model in their rate expressions in analyzing nitrification results. Applying the expression to the ammonia oxidation by Nitrosomonas, the  $K_{\rm S}$  values determined were small (0.2 to 1.7 mg/l NH $_4^+$ -N, Knowles, Downing and

Barrett, 1975). A greater range of  $K_s$  values have been reported for <u>Nitrobacter</u> but generally indicate that the oxidation reaction can be considered independent of  $NO_2^-$ -N.

The maximum growth rate coefficient (u\*) represents the mass of organisms produced per unit mass of organisms per unit time when growing at non-limiting substrate concentrations. A wide range of values of the coefficient for both <u>Nitrosomonas</u> (Loveless and Painter, 1968) and <u>Nitrobacter</u> (Boon and Laudelot, 1962) have been reported. The difference in culture conditions (pH, temperature, etc.) may account for the varying results. The substrate utilization rate (K) can be related to the growth rate coefficient by consideration of the yield coefficient (Y):

$$K = \frac{u^*}{Y} \qquad .....(5)$$

Using reported values of actual cell yields and  ${\rm C_5H_7O_2N}$  as an empirical cell formula for nitrifying bacteria, Haug and McCarty (1972) proposed the following overall mass balances combining nitrification assimilation:

Nitrosomonas 55 NH<sub>4</sub><sup>+</sup> + 5 CO<sub>2</sub> + 76 O<sub>2</sub> + 
$$C_5H_7O_2N + 54 NO_2^- + 52 H_2O + 109 H^+$$
 .....(6)

Nitrobacter 400 NO<sub>2</sub><sup>-</sup> + 5 CO<sub>2</sub> + NH<sub>4</sub><sup>+</sup> + 195 O<sub>2</sub> + H<sub>2</sub>O +  $C_5H_7O_2N + 400 NO_3^- + H^+$  .....(7)

On the basis of the equations, 20 mg of ammonia nitrogen would produce only 3 mg of <u>Nitrosomonas</u> and approximately 0.5 mg of <u>Nitrobacter</u>. These yields are less than 10 percent of that normally observed for heterotrophic bacteria.

The growth of microorganisms may also be expressed in terms of their doubling or generation time  $(t_d)$ . This coefficient is related to the substrate utilization rate (K) according to:

$$K = \ln 2/t_d Y \qquad \dots (8)$$

Substituting appropriate K and Y values it is found that the generation times of the autotrophic nitrifying bacteria are in the range of 10 to 30 hours. Generation times for heterotrophic bacteria are frequently reported as 20 to 40 minutes. In a suspended growth or activated sludge system, a direct consequence of the slow growth rate or long generation time of nitrifiers, is the requirement to provide a sufficient solids retention time (SRT) or sludge age to retain an adequate population of these organisms. Solids retention time is a measure of the average retention time of the bacterial cells in the system. The SRT and the growth rate of organisms in an activated sludge plant are related according to:

$$SRT = \frac{1}{u}$$
 ....(9)

SRT is normally defined as the total mixed liquor suspended solids under aeration divided by the daily solids lost in the effluent or through sludge wasting.

## **Environmental** factors

Autotrophic nitrifying organisms are obligate aerobes. Numerous reports indicate that in order to ensure that dissolved oxygen is not a limiting nutrient for nitrification, a level not less than 2.0 mg/l must be maintained (Wuhrmann, 1963, Painter, 1975). The stoichiometric oxygen requirements based on equations 1 and 2 are 3.43 mg oxygen/mg  $NH_4^+$ -N and 1.14 mg oxygen/mg  $NO_2^-$ -N. Jeffrey and Morgan (1959) found that oxygen uptake values in BOD tests for nitrification were within 2.5 percent of the theoretical values.

Nitrification, like most bacterial processes, is affected by pH conditions. Generally optimum conditions have been found to exist between pH 8.0 and 9.0 (Figure 2). Variations in pH optima could be due to shock effects in adjusting culture conditions or improper acclimation. Haug and McCarty (1971) using a submerged aerobic filter found that the rate of nitrification at pH 6.0 approached rates at higher pH conditions (7 to 8.5). after an acclimation period of approximately 10 days.

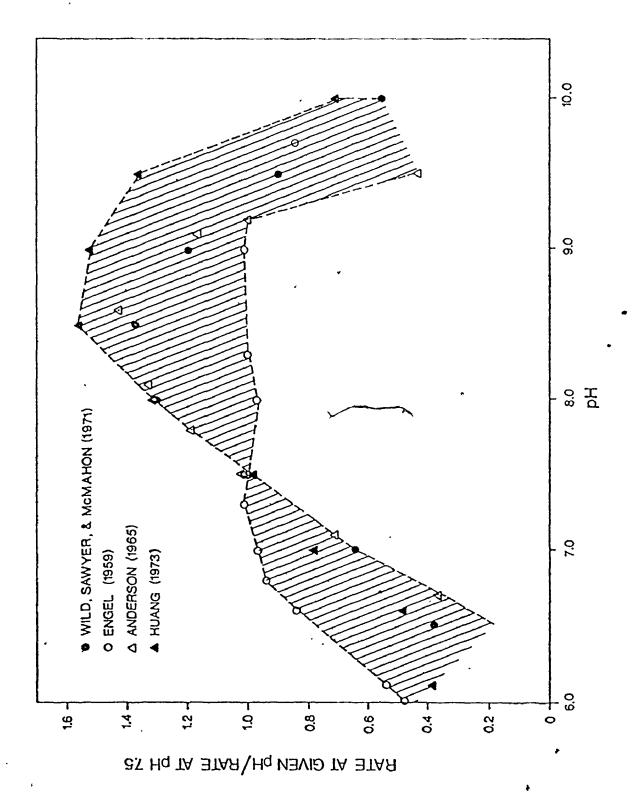


FIG. 2 EFFECT OF PH ON NITRIFICATION RATE

Nitrification is a hydrogen ion producing reaction (equation 1). Since pH values less than 6.0 would likely affect the nitrification rate the alkalinity of the waste is an important consideration. Assuming the pH is less than 8.5, the hydrogen ions produced during nitrification react with the bicarbonate in the wastewater, resulting in an increase in  $\rm CO_2$  concentration and a decrease in bicarbonate alkalinity according to:

$$H^{+} + HCO_{3}^{-} \rightarrow CO_{2} + H_{2}O$$
 .....(10)

Based upon equations 1 and 10; approximately 7 mg of bicarbonate alkalinity, expressed as  $CaCO_3$ , are required to neutralize the hydrogen ions produced during the oxidation of 1 mg of  $NH_4^+$  -N. Calculations involving carbonic acid equilibria show that for wastewater with an alkalinity of 200 mg/l as  $CaCO_3$ , approximately 20 mg/l of  $NH_4^+$ -N could be oxidized before the pH dropped below 6.0 if all the  $CO_2$  produced remained in solution (Haug and McCarty, 1971). In most nitrifying reactors the  $CO_2$  is stripped from solution tending to help maintain a neutral pH. In operating a rotating biological contactor for carbon (BOD) removal-nitrification of a municipal sewage with moderate alkalinity (approximately 120 mg/l as  $CaCO_3$ ), Wilson (1975) found that 15 to. 20 mg/l  $NH_4^+$ -N were nitrified and the pH was never less than 6.8.

The process of nitrification occurs over a range of approximately  $4^{\circ}$  to  $45^{\circ}$ C with optima at about  $35^{\circ}$ C for <u>Nitrosomonas</u> (Buswell, Shiota, Lawrence, and Meter, 1954) and  $35^{\circ}$  to  $42^{\circ}$ C for <u>Nitrobacter</u> (Deppe and Engel, 1960, Laudelot and Van Tichelen, 1960). It has been shown to be strongly dependent on temperature. In a suspended growth nitrification system differences in the reported temperature sensitivity (Figure 3) may be due to differences in reactor SRT. Increasing SRT results in a decrease in the temperature sensitivity. Supported growth systems (trickling filter, rotating biological contactor, etc.), which could be expected to be operating at high SRT's, show reduced temperature sensitivity for nitrification (Wilson, Murphy, Sutton, and Jank, 1975, Huang, and Hopson, 1974).

Nitrifying organisms, especially <u>Nitrosomonas</u>, are susceptible to a number of inhibitors which may be present in municipal and industrial wastewaters. A number of metals are toxic to nitrifiers but the concentration required to cause inhibition is dependent on the state of the culture. Copper,

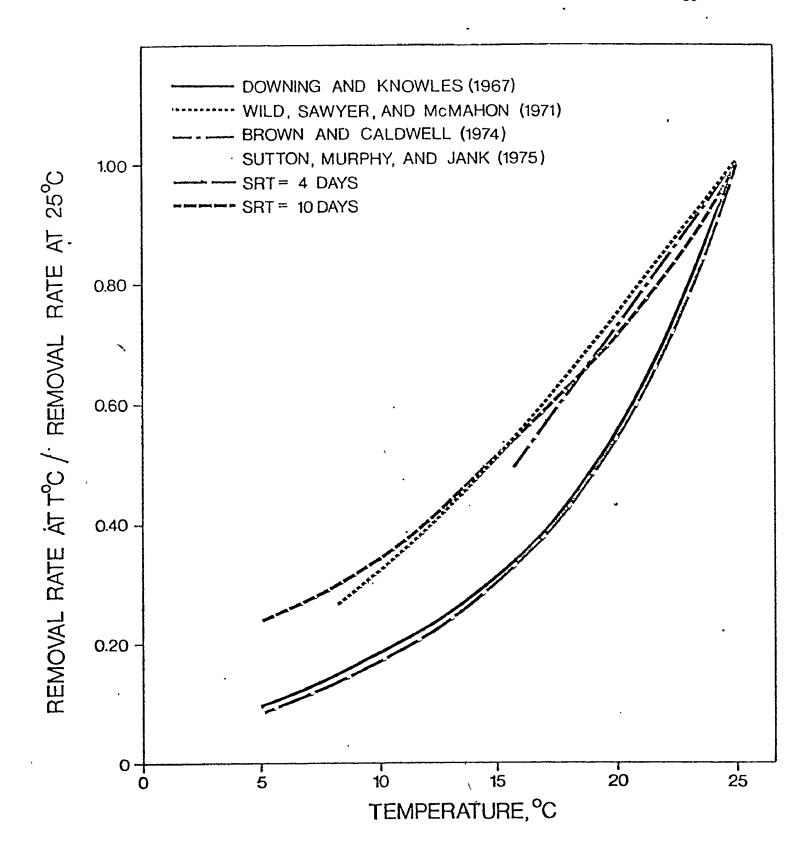


FIG. 3 EFFECT OF TEMPERATURE ON NITRIFICATION RATE

silver, mercury, nickel, chromium, and zinc all may inhibit nitrification under certain conditions. Downing, Tomlinson, and Truesdale (1964) identified a large number of organic materials which caused a reduction in nitrification rates in suspended growth systems. Thiourea, phenol, phenolic compounds, cresol, and halogenated solvents were some of the potential inhibitors identified. In screening the organic materials as nitrification inhibitors, unacclimated organisms were used and therefore the results may not reflect what actually would occur in a treatment plant.

Substrate and product inhibition has been reported for nitrification. Concentrations greater than 2500 mg/l nitrite-nitrogen have been found to inhibit Nitrosomonas (Meyerof, 1916, Lewis, 1959). At 1400 mg/l nitrite-nitrogen the growth of Nitrobacter has been affected (Boon and Laudelot, 1962). Such conditions are unlikely to be encountered in the treatment of municipal wastewaters but could occur in the treatment of nitrogenous industrial wastes.

## Carbon Removal-Nitrification Treatment Alternatives

The process reactors available for carbon removal (BOD) - nitrification can be classified according to the nature of their biological growth. Activated sludge systems can be regarded as suspended growth reactors whereas systems in which growth occurs on, or within a solid media, can be termed supported growth reactors.

The two basic process schemes available for nitrification in the activated sludge process are the combined sludge process and the separate sludge process. In the combined sludge process, carbon removal and nitrification are carried out using the same sludge. In the separate sludge process, the biological reactions are carried out by different microorganisms in separate reactors.

A number of combined and separate sludge treatment plants have recently been designed to provide conditions for carbon removal and nitrification. Design has been based on criteria developed from studies during which wide variations and fluctuations in the nitrification efficiency were obtained. Mulbarger (1971), in the only intensive comparative experimental assessment of a combined and separate nitrification system to date, stated in reference to his experimental results that "Firm design and operational recommendations cannot be made at this time." Based on

daily composite sampling and nearly steady flow of a municipal sewage, he did find that his combined sludge system failed to exhibit the same degree of soluble effluent TKN stability as the separate system. More recently Lawrence and Brown (1973) concluded that nitrified effluents of essentially identical quality could be produced from separate and combined sludge systems based on limited data from a pilot plant study.

Although nitrification has been obtained for many years in combined sludge systems similar to the configurations previously noted (Figure 1), the reported results indicate little more than conditions necessary to achieve nitrification. Furthermore, there are many contradictions in the literature regarding these conditions. Beckman, Avendt, Mulligan, and Kehrberger (1972) utilizing laboratory and full-scale treatment facilities found at 10° to 18°C, sludge ages greater than six days were required for optimum nitrification. Wuhrmann (1968) stated that at a temperature above 14°C, a sludge age of two to three days will yield a stable nitrification system. Prakasam and Loehr (1972) found complete nitrification at sludge ages greater than three days in treating a high nitrogen strength poultry waste at 20°C.

Several investigations have been made to determine the feasibility of nitrification in the separate sludge system. Wild, Sawyer, and McMahon (1971) studied batch nitrification in the separate sludge unit. A continuous nitrification unit operated under controlled conditions was used to provide sludge to conduct the batch studies on the effects of temperature and pH on the nitrification rate. Barth and Brenner (1968), operating at 185 gpd three-stage N removal pilot plant, obtained an average 87% conversion of reduced nitrogen compounds to nitrate in the separate nitrification stage. Operation was over a temperature range of 12° to 22°C and at an average solids retention time of 22 days.

Although optimization of separate sludge systems has not been performed to any great extent, as reflected by the wide variations in efficiency, the high values frequently obtained indicate the potential and encourage further research.

A limited number of supported growth systems have been utilized for carbon removal and/or nitrification including trickling filters, submerged aerobic filters, and rotating biological contactors.

## PROCESS MODELLING FOR NITRIFICATION SYSTEMS

## Pseudo "Steady-State" Modelling

Process design requires rate data to be expressed as parameters which are useful from an engineering point of view. In order to describe the substrate removal rate in any biological waste treatment process an overall kinetic expression compatible with the fundamentals of microbiology, kinetics, and transport phenomena is necessary.

For a given reaction environment, substrate removal rate is a function of the concentration of substrate (S) and active biological solids (X):

$$\frac{dS}{dt} = f(X,S),$$

$$\frac{dS}{dt} = KXS^{Z}, \qquad ......(11)$$

where K is the substrate removal velocity, and the unit rate of substrate removal can be expressed as:

$$\frac{dS}{Xdt} = KS^{Z} \qquad \dots (12)$$

This normalization procedure has been used by numerous authors (Wuhrmann and Mechsner, 1965, Busch 1971), and in fact forms the basis for most engineering design work in biological treatment. Although strictly empirical the unit rate concept is fundamentally related to biological growth kinetics.

The relationship between substrate removal and biological growth can be expressed by:

$$\frac{dX}{dt} = \gamma \frac{dS}{dt} \qquad (13)$$

where  $\frac{dX}{dt}$  = net growth rate of microorganisms (M/T), and

Y = apparent yield coefficient.

Using the monod kinetic model to represent the kinetics of biological growth:

$$u = \frac{1}{X} \frac{dX}{dt} = \frac{u * S}{Ks + S} \qquad (14)$$

where  $u = \text{net growth rate } (T^{-1}),$ 

 $u^*=$  maximum growth rate coefficient  $(T^{-1})$ ,

S = substrate concentration (M/L<sup>3</sup>), and

Ks= substrate concentration at one half maximum growth rate  $(M/L^3)$ .

Combining equations 13 and 14:

$$\frac{1}{X} \frac{dS}{dt} = \frac{1}{Y} \frac{u*S}{Ks+S} \qquad (15)$$

At low Ks values this expression becomes:

$$\frac{1}{X} \frac{dS}{dt} = \frac{u^*}{Y} = K \qquad ....(16)$$

Under such conditions the model is zero order independent of substrate concentrations. The zero order nature of the nitrification reaction, down to very low  $NH_4^+$ -N values, has been illustrated by numerous researchers (Wild, Sawyer, and McMahon, 1971, Huang, 1973, and Kiff, 1972). It is reasonable to anticipate that the reaction rate will be zero-order with respect to substrate concentration at all practical  $NH_4^+$ -N levels allowing the unit rate of removal to be expressed as a constant.

In order to relate the unit rate of ammonia removal to biological growth kinetics a measure of the active nitrifier population (X) is necessary. Techniques for determination of the autotrophic nitrifier fraction in a heterogeneous biological system are presently being developed (Srinath, Prakasam, and Loehr, 1974). The critical factors affecting the

autotrophic-heterotrophic population dynamics in an activated sludge system are the solids retention time and influent wastewater organic carbon to inorganic nitrogen (C/N) ratio. Combined sludge systems operating under similar environmental conditions (reactor pH, dissolved oxygen and temperature) will contain the same number of nitrifiers at equal SRT's. Combined sludge systems operating under similar environmental conditions will contain the same fraction of nitrifiers at equal SRT's and influent wastewater C/N ratios.

In order to maintain consistent ammonia removal or nitrification under pseudo "steady-state" conditions the fractional increase in Nitrosomonas during aeration must be greater than or equal to the corresponding fractional increase in sludge age. The growth rate of Nitrosomonas and consequent rate of ammonia removal can be strong function of temperature. Knowles, Downing, and Barrett, (1965) claimed that the maximum specific growth rate of Nitrosomonas was increased by approximately 9.5 percent for each degree celcius increase in temperature. For many reactions, the variation of rate with temperature under pseudo "steady-state" conditions may be represented by an Arrhenius relationship. This relationship can be represented as:

 $K = Ae^{-E/RT}$ , ....(17)

where

 $K = reaction rate constant (day^{-1})$ 

A = frequency factor  $(day^{-1})$ 

E = activation energy (cal g-mole<sup>-1</sup>)

 $R = universal gas constant (cal g-mole^{-1} oK^{-1})$ 

T = temperature (OK)

Because temperature is a parameter that varies throughout the year in nearly every treatment plant, its effect on nitrification is important. In a particular operating alternative, the solids retention time may have to be significantly increased to compensate for low temperatures (i.e. below 10°C). If a plant does not have this capability, nitrification may be partially or completely lost.

## Dynamic Modelling

The effects of fluctuations in many variables on the dynamic behaviour of nitrification alternatives cannot be predicted from pseudo "steady-state" parameter values. A sudden rainfall may result in a hydraulic impulse to a treatment plant. Little insight into the expected profile of recovery and time to return to normal operation would be gained by examination of the pseudo"steady-state" process design parameters.

While dynamic models for the activated sludge processes have been and are being developed (Blackwell, 1971, Smith and Eilers, 1970), these models do not extend to systems for carbon removal-nitrification. In one study, Downing, Painter, and Knowles (1964) used batch nitrification data and computer techniques to model the time dependent results. Lijklema (1973) used a limited amount of steady-state literature data to substantiate his model for nitrification in a single-stage activated sludge process. Poduska (1973) presented the most extensive dynamic study of the nitrification process. Using a synthetic feed and bench scale apparatus, the transient responses of the nitrifying culture were investigated for various dynamic forcings at constant temperature. The dynamic model developed considered only a single-stage system and did not account for the presence of heterotrophic bacteria and subsequent organic carbon degradation.

In assessing the effectiveness of the combined and separate sludge systems under non-steady operating conditions the variables of concern are fluctuations in hydraulic, organic, and inorganic loading, and variations in raw wastewater pH, and the temperature levels. By quantifying the effect of these variables an evaluation of the stability of the nitrification alternatives will be afforded.

The pseudo "steady-state" effect of temperature on many biological systems has often been represented by Arrhenius type expressions. These expressions could not be expected to predict the transitional or short term response to sudden changes in temperature. Similarly the transitional or short term response of biological systems to pH changes could not be expected to be predicted by pseudo "steady-state"

expressions (Downing, and Knowles, 1967). Little or no information exists in the literature regarding transitional responses to changes in temperature and pH, on combined and separate sludge systems operated for carbon removal-nitrification.

When adequate experimental facilities are available, qualititative comparisons can be made between process alternatives. The wastewater feed can be varied in temperature and pH level, in organic and inorganic concentration, and in flow rate, and the resulting effluent response observed. Mathematical modelling and simulation, and conventional time series or spectral analysis are some basic tools by which process response can be interpreted quantitatively.

## Deterministic dynamic models

The purpose of seeking a deterministic or mechanistic model is to allow an explanation for the response of a process to variations in the input variables. If such models can be determined for the combined and separate sludge carbon removal-nitrification alternatives, the response of the processes to input variables which cannot be examined experimentally due to equipment or time limitations, can be examined. Deterministic models nearly always have empirical qualities and it is unlikely that a system as complex as a biological wastewater treatment plant will ever be described exactly be a theoretical model.

Through consideration of the following an attempt can be made to develop a mechanistic model for the activated sludge process, operated for nitrification:

- 1) the flow regime in the aeration basin,
- 2) the oxygen requirements of the system,
- 3) the representation of the performance of the final settler.
- 4) the effect of secondary parameters such as wastewater temperature, pH, and toxic substances,
- 5) the form of the equation expressing the time rate of synthesis of substrate into new cells, and
- 6) such factors as cell lysis, basal metabolism, and solubilization of particulate degradable organic carbon.

In modelling, the separate and combined sludge systems described here (Figure 1) ordinary differential equations are used to express the dynamic behaviour of each of the following: heterotrophs, Nitrosomonas, Nitrobacter, degradable organic carbon, ammonia nitrogen, nitrate nitrogen, and nitrite nitrogen. It is assumed that all the heterotrophic organisms can be represented by a characteristic organic carbon substrate utilization rate described by a linear correlation. The available carbon substrate is that measured as filterable degradable organic carbon. Substrate utilization by the nitrifying bacteria is assumed to follow Monod kinetics. One of the powerful advantages of Monod kinetics is that it is a continuous function reducing to first order at low substrate concentrations and approaching zero order at high substrate concentrations. One inherent weakness of the Monod function often sited (Powell, 1967) is that it shows the growth rate to be only dependent on the instantaneous substrate concentration exhibiting no lag in response to changes in concentration. The lag in growth rate exhibited by nitrifying bacteria for changes in ammonia and nitrite concentrations has been shown by Hofman and Lees (1953), Lees and Simpson (1957), and Laudelot and Van Tichelen (1960) to be very small.

In the expressions for the growth rate and consequent substrate utilization equations, no account will be given for variations in dissolved oxygen (DO), temperature or pH. The parameter values in these equations will be assumed constant reflecting constant DO, temperature, and pH.

In the organism material balances, reduction in cell growth due to lysis or endogeneous respiration is represented by a combined decay coefficient for each of the organism groups involved.

While, in developing the dynamic models, no attempt will be made to express the kinetic growth coefficients as functions of the transient loading condition, this assumption, as applied to the yield coeffficient for heterotrophs, is questionable and a relation expressing the variation with loading may be necessary (Smith and Eilers, 1970). The small yield values reported for nitrifiers (Painter, 1970) makes any correction for ammonia and nitrite loading unnecessary. Similarly the insignificant yield value for Nitrosomonas allows the neglection of the

4

small amount of ammonia used in cell synthesis that does not appear as nitrite. The remaining coefficients, the linear coefficient describing heterotrophic growth and the maximum growth rate and saturation coefficients in the Monod models for nitrification, will be assumed constant over a given transient period. Providing the dynamic investigations involve relatively short term temporal periods, the possibility for changes in these parameters should be small.

In the initial model development, each aerator will be assumed completely mixed. Residence time distribution studies, may indicate that a different hydraulic model is necessary.

An adequate representation of the performance of the final settler is a complication in any model for the activated sludge system. Since the interactions between the aeration basin and the solid-liquid separators are not of prime importance in this research, we will consider the settlers to be zero volume devices whose efficiency can be described by a separation coefficient determined experimentally. No bacterial growth or changes in organic carbon, ammonia, nitrite, and nitrate are assumed to occur in the settler and the return flow and effluent streams contain equal proportions of all biological groups.

The dynamic model describing the single-stage combined sludge carbon removal-nitrification alternative is represented in Figure 4. In this figure:

V = volume of reactor (L<sup>3</sup>),

 $F = flow rate (L^3/T),$ 

S = limiting substrate concentration (M/L<sup>3</sup>),

 $X = \text{organism concentration } (M/L^3),$ 

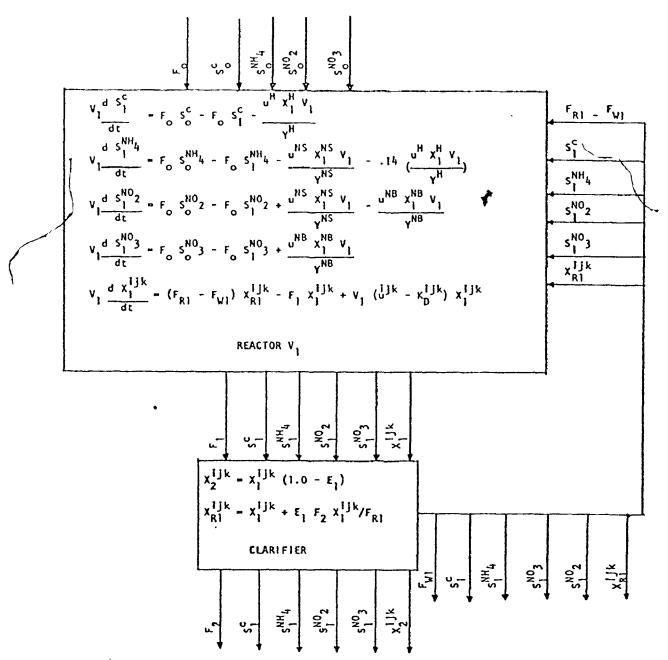
0,1,2,3,4,R1,W1 = notations which relate to flow streams,

H,NS, NB = notations which relate to heterotrophs,

Nitrosomonas, and Nitrobacter, and

 $C,NH_4,NO_2,NO_3$  = notations which relate to filterable degradable organic carbon, ammonia, nitrite, and nitrate.

In the model development, substrate and organism material balances are presented assuming the nitrification reactions follow the stoichiometric equations 1 and 2 and the general material balance for completely mixed reactors applies:



1, J, k - represent heterotrophs, <u>Mitrosomonas</u>, and <u>Mitrobacter</u>

FIG. 4 MODEL REL'ATIONSHIPS FOR SINGLE-STAGE COMBINED SLUDGE SYSTEM

rate of material accumulation

rate of material flow into réactor

rate of material
flow out of
reactor

rate of appearance

+ or disappearance of
material due to
growth or utilzaation by organisms

Details concerning the material balances are presented in Appendix A.

Similar models involving substrate and organism balances can be developed for the TSC and TSS sludge systems.

Before the equations which comprise the proposed dynamic models can be solved, values for the various parameters must be determined. Methods are discussed in Append $\hat{x}$  A for determining the parameter values.

Having developed a dynamic mathematical model, the equations which comprise the model must be solved in order to predict behaviour with respect to time. Simulation packages which can be used in solution of the differential and algebraic equations comprising the dynamic models include DYNSYS AND CSMP. DYNSYS is a modular dynamic system simulation developed by A.I. Johnson and associates at McMaster University. Using this modular routine, a particular process can be simulated by assembling a number of modules representing the individual processes within the plant. Both differential and algebraic equations can be handled. CSMP (IBM, 1972) is an equation oriented approach differing from the modular approach in that equations are not integrated one at a time but simultaneously. CSMP is highly user oriented allowing concentration on model development and simulation results rather than on details of the computations.

# Linear dynamic stochastic models

The activated sludge system is a heterogeneous culture system containing a wide spectrum of organisms interacting and competing for the many organic and inorganic substrates. In describing the single-stage combined sludge (SSC) carbon removal-nitrification alternative, the heterogeneous system was simplified by considering independent organism groups growing on defined substrates. A number of other assumptions are inherent in the model development to reduce the complexity of the process including:

- the neglection of synergism or antagonism resulting from organism and substrate interaction,
- 2) the utilization of empirical functions to describe biological growth and consequent substrate removal,
- 3) the neglection of any lag in growth rate exhibited by both autotrophic and heterotrophic organism groups in response to changes in substrate concentration.
- 4) the assumption that kinetic growth coefficients are constant, unaffected by variations in loading conditions to the treatment plant, and variations in D.O., temperature, and pH within the aeration basin, and
  - 5) the neglection of the effect on substrate removal of any physical-chemical mechanisms such as adsorption.

It is apparent that the highly complex nature of the activated sludge process makes it most unlikely that a "true" unified model, allowing extrapolation of process performance from one system to another, could be developed. The obscure, undefined processes will lead to output results not accountable by the model. Even if the proposed model for the SSC activated sludge system, with its stated weaknesses, was sufficient to describe the carbon removal-nitrification performance, there results a considerable number of time dependent growth and substrate removal functions complicating the usefullness of the model.

When dealing with systems that do not behave according to a relatively simple deterministic model and are complicated by unexplainable output variation or noise it may be more useful to utilize linear dynamic-stochastic models. The activated sludge system may well tend to act as a linear system because of a central limit or averaging effect of the many growth and substrate removal functions controlling the process. Consequently, linear transfer function models should describe the time-dependent behaviour adequately. Such models, together with linear time series models to account for the unexplained output variations, can be used to describe the response of the combined and separate sludge systems to input variations.

It is accepted that the levels of the inputs, such as concentration of NH<sub>3</sub>-N and organic carbon to the alternative carbon removal-nitrification systems (Figure 1) will result in a delayed response in the output levels. Such a change is referred to as a dynamic response and a model describing this dynamic response is referred to as a transfer function model.

These models are of the form:

$$Y_{t}=\Sigma_{i=1} \frac{(\omega_{0}-\omega_{1}\beta...-\omega_{s}\beta^{s}i)}{(1-\delta_{1}\beta...-\delta_{r}\beta^{r}i)} X_{i},t-b \qquad ..... (18)$$

$$=\sum_{i=1}^{V} i^{(\beta)X}i, t-b$$

6

where	Yt	= output deviation from the mean at time t,
	ω,δ	<pre>= model parameter values,</pre>
	r,s	= model orders
	b	= delay period,
	X <sub>it</sub>	= the deviation from the mean of the i <sup>th</sup>
	10	variable at time t,
	β	= backward shift operator, and
	V <sub>;</sub> (β)	= discrete transfer function model relating
	,	$Y_{t}$ and $X_{i,t}$

Other influences which affect the output levels, referred to as noise, can be taken into account by a noise model:

where N<sub>t</sub> = disturbance in the output due to all sources other than the X<sub>it's</sub> = model parameter values, p,d,q = model orders, and

Therefore, the combined transfer function-noise (TF-N) model can be written:

The building of these models is accomplished by an iterative procedure involving:

- 1) identification of the transfer model polynomial orders  $(r_i, s_i i=1, 2...)$  and delay period (b),
- estimation of the parameters of the tentatively identified transfer function model,
- 3) identification of the noise model,
- 4) re-estimation of the parameters of the combined transfer function-noise model, and
- 5) diagnostic checking of the fitted model to verify adequacy.

In the identification stage of the transfer function model building sequence, cross correlation techniques may be used to indicate r,s, and b and estimate the impulse response values  $(V_i)$  of the transfer function model. The cross correlation function between two input-output series (x and y) separated by a constant interval or lag (k) is given by:

$$\rho_{xy}(k) = \frac{\gamma_{xy}(k)}{\rho_{x}\rho_{y}} \qquad k = 0, \pm 1, \pm 2 \dots (21)$$

where

$$y_{xy}(k)$$
 = cross covariance function between x and y, and

$$\rho_{x}$$
 and  $\rho_{y}$  = constant standard deviations of the x and y series.

Determination of the cross correlation functions also allows a preliminary assessment of which input and output characteristics are most related.

In place of using cross correlation techniques to estimate the  $V_i$  values of the transfer function model, they can be determined directly through a relationship with the autocovariance function of the input series and the cross covariance function between the input-output series (Box and Jenkins, 1970a). Once  $V_i$  values are obtained, preliminary estimates for the model parameters  $(\omega,\delta)$  can be determined through use of an identity to equation 18 (Box and Jenkins 1960b). Beginning with the preliminary estimates, an efficient estimation procedure is used such as non-linear least squares to determine the parameter values.

The noise component of the combined TF-N model is identified by examining the residuals of the transfer function model using autocorrelation techniques.

Once diagnostic checking has varified the adequacy of the fitted model, it is possible to forecast future values of the output series  $Y_{t+i}$  using  $X_t$  as a leading indicator. This is accomplished by using the "difference" form of the combined TF-N model (Box and Jenkins, 1970c).

#### EQUIPMENT AND PROCEDURES

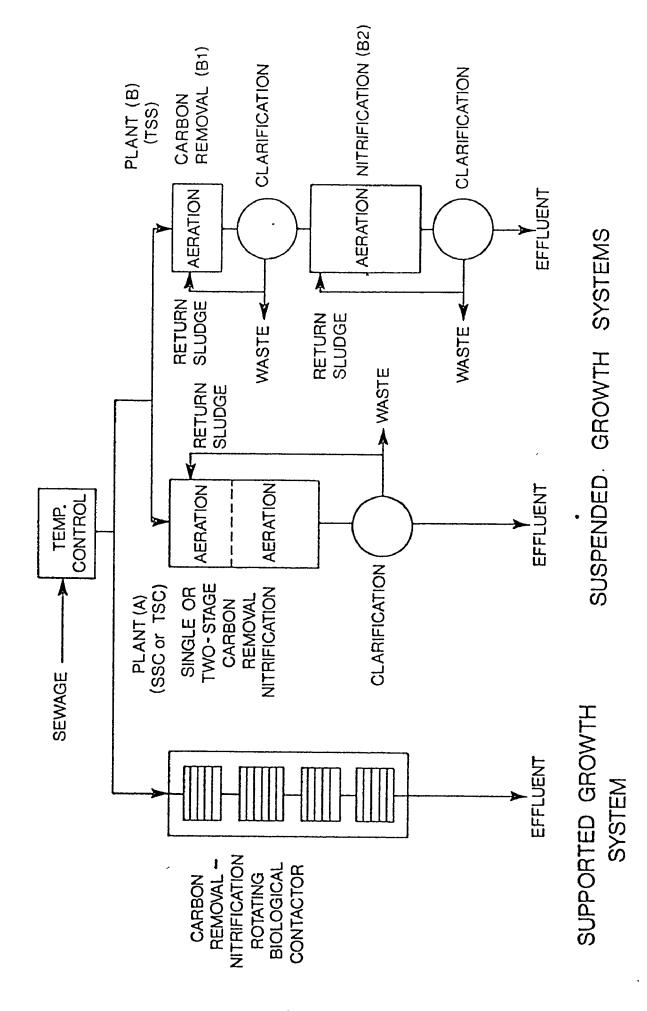
### Pilot Plant Design and Operation

To evaluate the combined and separate sludge carbon removalnitrification alternatives a pilot plant was designed by P.M. Sutton and K.L. Murphy and constructed at the Wastewater Technology Centre, Burlington, Ontario by Sutton and members of the pilot plant staff of the Environmental Protection Service. This facility, designed for a maximum hydraulic loading of 32.7 m<sup>3</sup>/day (7,200 lgpd), permits study of both suspended and supported growth carbon removal-nitrification systems (Figure 5).

The suspended growth systems for carbon removal-nitrification, designated as plants A and B, consists of two 2.81 m $^3$  (480 lg) plexiglass, dispersed air aeration tanks. Plant A contains a removable divider allowing either the two-stage or the single-stage combined sludge system (TSC or SSC) to be operated in parallel with Plant B. Plant B forms the two-stage separate sludge system (TSS), a fixed divider separating the reactor into two aeration tanks (Bl and B2). Three additional removable dividers were added to Plant B when required to form a five-stage combined sludge system (FSC). The circular clarifiers are 0.76 m (2.5 ft) in diameter providing a surface settling rate of 28.9 m $^3$ /day/m $^2$  (590 lgpd/ft $^2$ ) and a hydraulic detention time of 1.0 hr at a feed rate of 9.1 l/min (2.0 lgpm).

Variable speed positive displacement pumps deliver the wastewater to the reactors from a cooling unit capable of cooling to  $3^{\circ}\text{C}$  or heating to  $25^{\circ}\text{C}$  with a precision of  $\pm 1^{\circ}\text{C}$ . The biological solids retention time is controlled in the reactors by wasting from return sludge lines.

By control of the SRT and temperature, a pseudo "steady-state" operation point was approached. Using 24 hr time-averaged sampling, and parallel operation, the performance of any two alternatives could be assessed. In order to determine the kinetic removal rates used to describe the pseudo "steady-state" performance of the nitrification systems, the effluent from the reactors must contain residual filterable ammonia. When complete ammonia conversion was obtained, additional amounts of ammonia (NH<sub>4</sub>Cl) were added as the nitrification rate was considered to be substrate



SUSPENDED AND SUPPORTED GROWTH CARBON REMOVAL - NITRIFICATION PILOT PLANT, FIG. 5

limited at effluent  $NH_4^+$ -N values less than 1.0 mg/l. Bicarbonate (NaHCO $_3$ ) was added to the feed when necessary to avoid any alkalinity limitation on the ammonia removal rates. The dissolved oxygen in the reactors was maintained above a minimum level of 2.0 mg/l.

To assess the dynamic behaviour of the carbon removal-nitrification alternatives, the input variables had to be artificially disturbed. The degritted raw municipal wastewater pumped from the Burlington Skyway treatment plant contained normal diurnal variations in organic and inorganic concentrations (Table 2). The pilot plant variable speed pumps provided a means of delivering the wastewater to the reactors in any hydraulic pattern. This, together with supplementing the wastewater with organic carbon (dextrose) and inorganic nitrogen (NH $_4$ Cl), allowed any combination of hydraulic, organic, and inorganic loading to be obtained.

### Experimental Plan

The pilot plant program was divided into four phases covering the period from May 1973 to June 1975. Phase I consisted of the design and construction of the pilot plant facilities extending from May until October, 1973.

Start-up of facilities and preliminary experiments constituted Phase 2. Initially mixed-liquor suspended solids from the Burlington Skyway activated sludge plant were added to the aeration sections of Plants A and B (Figure 5) and continuous operation began immediately. The preliminary experiments which followed determined the operating limitations of the carbon removal-nitrification alternatives, allowing formulation of a concise experimental program designed to compare the systems under pseudo "steady-state" conditions. This experimental period designated Phase 3 extended from November 1973 until February 1975. During Phase 3 preliminary non-steady and variable loading experiments were conducted.

Phase 4 concluded the experimental program and involved experiments designed to assess and compare the dynamic behaviour of the carbon removal-nitrification systems. During this phase, additional pseudo "steady-state" experiments were completed.

TABLE 2 RAW WASTEWATER CHARACTERISTICS

Characteristic mg/l	Mean	90%*
COD	325	460
BOD <sub>5</sub>	120	200
Filterable Organic Carbon (FOC)	28	37
SS	240	450
Filterable NH4-N	. 15	. 18
Filterable TKN	17	25
Alkalinity (as CaCO <sub>3</sub> )	118	130

Note: \*90% of observations are equal to or less than stated value

## Pseudo "steady-state" experimental design

. The primary factors considered to affect the pseudo "steady-state" performance of the nitrification alternatives in treating municipal wastewater were the solids retention time (SRT) or sludge age and temperature. Three levels of system SRT and five levels of temperature were selected (Table 3). The SRT levels were determined on the basis of practical design considerations for full scale facilities. The temperature levels reflect values under which treatment plants may operate in continental climates.

The system SRT was determined by a combination of hydraulic residence time and intentional solids wasting. Details concerning the calculation of the system SRT for each alternative appear in Appendix B. The experimental design, formulated to compare the operating alternatives (Table 4), allowed for a complete assessment of the two-stage separate sludge system (TSS) and a comparative assessment, through statistical methods, of the other alternatives (TSC and SSC). Additional pseudo "steady-state" experiments were undertaken to increase the precision of certain design parameter values derived for each alternative.

Pseudo "steady-state" was achieved by operating the reactors at the predetermined levels of SRT and temperature. After a sufficient period for temperature acclimation (Benedict and Carlson, 1973) and a minimum time of one and normally two or three system SRT's, the performance of the operating systems was assessed.

## Non "steady-state" experimental design

To assess the dynamic behaviour of the combined and separate sludge systems, the parallel TSC and TSS systems were subjected to a variety of non-steady conditions (Table 5). Previous to each experiment the systems were operated at solids retention times of three to four days and at a temperature of approximately  $15^{\circ}$ C, conditions critical for nitrification, to ensure measurable responses would be observed. To assess the effect of hydraulics on combined sludge systems operated for carbon removal-nitrification under non-steady conditions the single-stage system (SSC) was compared to the five-stage system (FSC) in experiment D9 (Table 5).

TABLE 3 EXPERIMENTAL DESIGN LEVELS

Factor	Operating Value	Design Level
System Solids Retention Time days	4 7 10	- 0 +
Temperature °C	5 10 15 20 25	1 2 3 4 5

TABLE 4 EXPERIMENTAL DESIGN FOR SYSTEM COMPARISON

Run No.	Temperature Level	System Solids Single-Stage Combined (SSC)	Retention Two-Stage Separate (TSS)	Two-Stage
PSS-15 -17 -01 -08	1	0 -	+ 0 0	+ 0
PSS-11 -21 -20 -57	2	0 +	- 0 0 +	- 0
PSS-13 -23 -25 -29	3	0 <del>-</del>	+ 0 0 -	<b>+</b> 0
PSS-44 -31 -34 -16	4	0 +	- 0 0 +	0
PSS-54 -53 -52 -61	5	0 -	+ 0 0 -	<b>+</b> 0
Repeats PSS-07 -14 -59 -58 -12 -60 -63 -49 -62 -55	1 2 2 3 3 4 4 5 5	- +	- + - - + -	+ + - + -

TABLE 5 NON "STEADY-STATE" EXPERIMENTS

Run No.	Bas Temp.	Baseline Conditions  np. Hydraulic Detention  Time (Aeration)  hrs	Run Length days	Experiment
נם	15	8	22	Step down in temperature of 5°C.
02	15	82	ω	pH impulse (HC1).
03	15	ω	4	Hydraulic impulse 2x baseline level for 10 hrs.(24-34 hrs).
04	15	∞ .	4	Hydraulic, plus OC, plus inorganic N impulse 2x mean levels for 10 hrs (24-34 hrs).
05	14	బ	0.	Designed changes in hydraulic, OC, and TKN levels.
90	13	<b>∞</b>	4	Hydraulic step 2x baseline level.
D7	13	బ	4	OC step addition of 40 mg/l C to normal levels.
+08	13	ω	4	Inorganic N step addition of 15 mg/l N to normal levels.
60 <b>*</b>	. 02	€	m	Hydraulic, plus OC, plus inorganic N impulse 2x mean levels for 10 hrs (24-34 hrs).
Note:	*SSRT +SSRT	2-3 days in SSC and FSC systems 6 days in TSS system,	systems.	

The high degree of correlation between the input variables, typical of the daily input to wastewater treatment plants, made it necessary to artificially disturb the inputs in a designed manner in order to properly assess their individual effects. Two levels of flow rate, filterable organic carbon, and filterable TKN were chosen (Table 6) and an experimental design formulated (Table 7). During this experiment (D5) the temperature and system SRT conditions were maintained at the baseline conditions.

### Sample Preparation and Analyses

Samples for organic carbon were prepared by filtration through 0.45 micron Gelman glass fiber filters followed by acidification to pH 2 with concentrated hydrochloric acid. Unfiltered TKN and COD samples were acidified with concentrated sulphuric acid.  $NO_3$ -N,  $NO_2$ -N, filtered TKN,  $NH_4^+$ -N and COD samples were prepared for analyses by filtration through 0.45 micron Gelman glass fiber filters. All samples were stored at  $O^0$  to  $O^0$  in polyethylene bottles while awaiting analyses except for BOD samples which were frozen.

The analytical procedures utilized are detailed in Appendix B.

TABLE 6 EXPERIMENT D5 DESIGN LEVELS

Influent Factors	Operating Value	Design Level
Flow Rate	9.0	+
l/min	4.6	-
Filterable OC	30 - 40 10 - 20	+
mg/l	10 - 20	-
Filterable TKN	20 - 30	+
mg/l	5 - 15	<del>-</del>

TABLE 7 EXPERIMENT D5 DESIGN

Time Period hrs	Flow Rate Level	Filterable OC Level	Filterable TKN Level
24 - 36	+	+	+
0 - 12	-	+	+
84 - 96	+	_	+.
36 - 48	- '	-	+
12 - 24	+	+	-
60 - 72	-	+	-
48 - 60	+	-	-
72 - 84	-	-	-
96 - 120	-	+	-
120 - 144	+	+	-
144 - 168	-		+
168 - 192	+	~	+
192 - 204	-	+	_
204 - 216	+	-	~
216 - 228	_	_	<b>-</b>
228 - 240	+	+	<b>-</b> ,

#### DISCUSSION OF RESULTS

#### Pseudo "Steady-State" Behaviour

Appendix B contains a computer listing of the complete day to day operating and analytical results during each phase of the experimental plan. The results of experiments, designed to compare the separate and combined sludge systems (Table 4), are tabulated in Appendix C (Table Cl). Included are the results of other pseudo "steady-state" experiments undertaken to increase the precision of the derived design parameter values. Any experiments repeated, at equal SRT and temperature conditions are considered "genuine" repeats (Appendix C).

## Evaluation of combined and separate sludge systems

The three carbon removal-nitrification systems were compared first in terms of their ability to remove filterable TKN. The results from the analysis of paired data over a system sludge age range of four to ten days and temperature range of 5° to 25°C using a "t-test" indicated comparable removal of filterable TKN by the separate (TSS) and combined sludge systems (TSC and SSC) at equal system solids retention times. While the data might appear to favour the separate sludge system (Figure 6), a "t" value less than the critical value (95% level) for the differences (Di values) in effluent TKN is indicated in Table 8. The lack of interaction between the differences in filterable TKN removal of the parallel operating systems and the solids retention time and temperature allowed the paired data to be statistically analysed as one data set.

The paired data from the operation of the TSS sludge system in parallel with either the SSC or TSC sludge system allowed a straight forward statistical assessment. In comparing the two combined sludge systems (SSC and TSC) such a procedure was not possible. The mean differences in treatment between the SSC and TSS sludge system and the TSC and TSS sludge system were determined. The two mean values were examined statistically to determine if they were significantly different from each other. Normal statistical procedures assume the populations from which the two means were derived, have equal variances. In this instance,

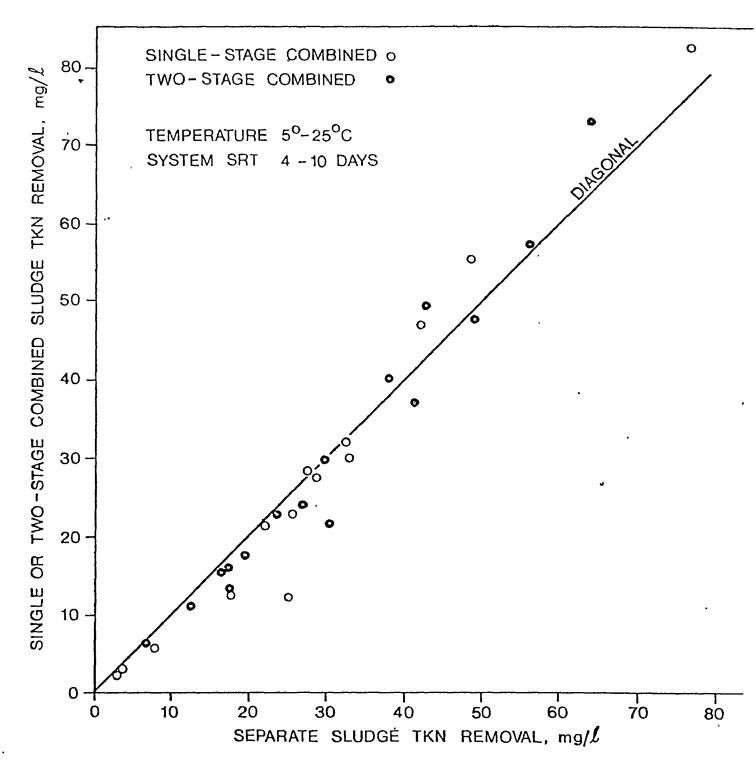


FIG. 6 FILTERABLE TKN REMOVAL IN COMBINED AND SEPARATE SLUDGE SYSTEMS

TABLE 8 DIFFERENCE IN EFFLUENT RESULTS FROM PARALLEL OPERATING SEPARATE AND COMBINED SLUDGE SYSTEMS

Parameter mg/l	D	SD/n	t	t α=.95	t α=.99
TSS vs SSC					
Filterable TKN NO3 + NO2-N SS Alkalinity (as CaCO3)	1.47 5.85	0.87 1.43 27.45 10.32	1.23 1.12	1.81	2.76 2.68
TSS vs TSC Filterable TKN NO <sub>3</sub> + NO <sub>2</sub> -N SS	0.51 0.33 10.50	1.09 0.66 35.61	0.41	1.76	
Alkalinity (as CaCO <sub>3</sub> )	2.87	21.90	0.61	1.76	
Note: $\frac{S_D^2}{n}$ = estimat	ed varian	nce of D	, where		
$S_D^2 = \frac{\Sigma(Di-D)}{n-1}$					
	fference te-combin				value *
n = number	of paired	data p	oints,	and ·	,
$t = \frac{\bar{D}}{S_D/(n)^{1/2}}$	•				

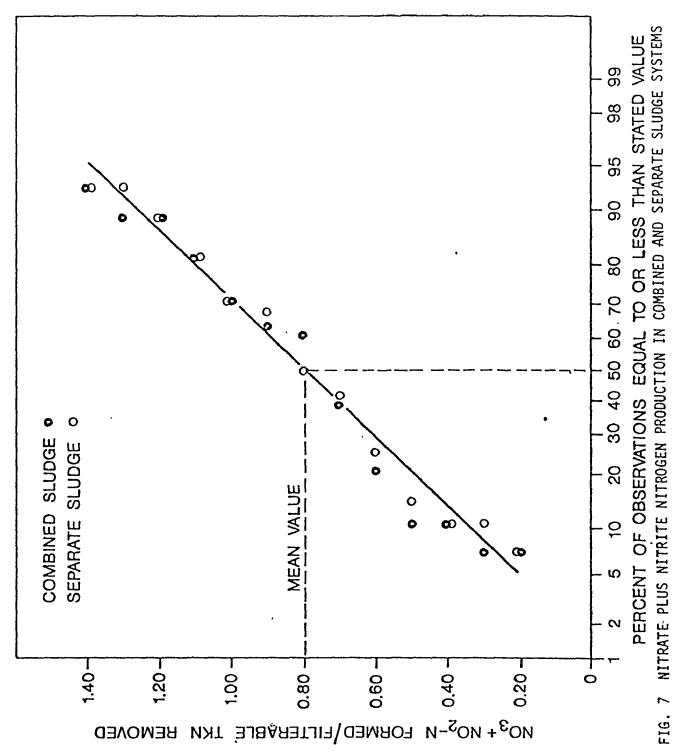
the ratio of the variances when compared to the "F" statistic indicated that such an assumption was warranted (Table 9) and allowed the use of a "t-test" for the comparison of the means. The resulting "t" value was not significant implying that equal filterable TKN removal was obtained with both SSC and TSC sludge systems (Table 9). This allows an evaluation of the effect of reactor configuration on nitrification. The use of dye studies to approximate the hydraulics of the systems (Appendix D) indicated that the single-stage reactor could be considered a completely mixed system characterized by a large value of the dimensionless dispersion parameter D/uL. The mixing regime in the TSC sludge system can be approximated by a value of D/uL equal to 0.2, closer to conditions characteristic of full-scale basins with large length to width ratios. The results and a summary of the procedures used in conducting the dye studies is presented in Appendix D. The lack of difference in filterable TKN removal in the two combined sludge systems (Table 9), with substantially different mixing regimes, supports the "zero-order" nature of the nitrification reaction down to very low filterable TKN or  $NH_{\Delta}^{+}$ -N values. This lack of concentration dependency is further demonstrated in Appendix C (Table C, Figure C1).

In biological wastewater treatment plants designed for nitrogen conversion and removal, denitrification will normally follow the nitrification process. An important variable in design of denitrification systems is the influent nitrate concentration. The paired data analysis indicated that the alternative carbon removal-nitrification systems will produce equal amounts of  $NO_3^- + NO_2^- - N$  (Tables 8 and 9). The nitrate production will be 0.8 g of  $NO_3^- + NO_2^- - N$  per gram of filterable TKN removed (Figure 7). The  $NO_2^- - N$  concentration in the reactor effluents will normally be less than 1.0 mg/l (Figure 8).

The hydrogen ions produced during nitrification, react with the bicarbonate in the wastewater resulting in a decrease in alkalinity. The analysis of paired data indicated that the SSC and TSC, and TSS sludge systems consumed equal amounts of alkalinity during the nitrification reaction (Tables 8 and 9). The geometric and arithmetric mean values for the consumption ratio are 3.3 and 3.9 grams of alkalinity (as CaCO<sub>3</sub>) consumed per gram of filterable TKN removed (Figure 9) considerably less than the theoretical value of approximately 7. The theoretical value assumes that no filterable TKN is used for assimilation of heterotrophic

DIFFERENCE IN EFFLUENT RESULTS FROM TSC AND SSC SLUDGE SYSTEMS TABLE 9

t α = .95	2.05	2.06	2.05	2.05					
. ب	0.40	0.41	0.57	0.19					
F α = .95	3.05	3.15	3.18	3.08					
S <sub>0</sub> 2	1.43	1.59	1.60	2.27					
S <sub>0</sub>	12.19	15.73	356.97 569.73	144.48 328.55					
٥١	-0.06	1.47	5.85	1.79		v		SC, and	ા
System Compared to TSS	SSC TSC	SSC TSC	SSC TSC	SSC TSC		number of paired data points	$\frac{1}{n_{SSC}}$ ) where,	ed variance for D <sub>TSC</sub> -D <sub>SSC</sub> ,	(SD TSC) + nSSC (SD SSC) nTSC + nSSC
Parameter mg/l	Filterable TKN	NO3 + NO2-N	SS	Alkalinity (as CaCO <sub>3</sub> )	Note: $S_0^2 = \frac{\Sigma(D1-D)^2}{n-1}$	n = number of	$t = \frac{D_{TSC} - \overline{D}_{SSC}}{Sp(\frac{1}{n_{TSC}} + \frac{1}{n_{TSC}}}$	Sp = pooled var	$\int_{0}^{2} \int_{0}^{1} \int_{0$



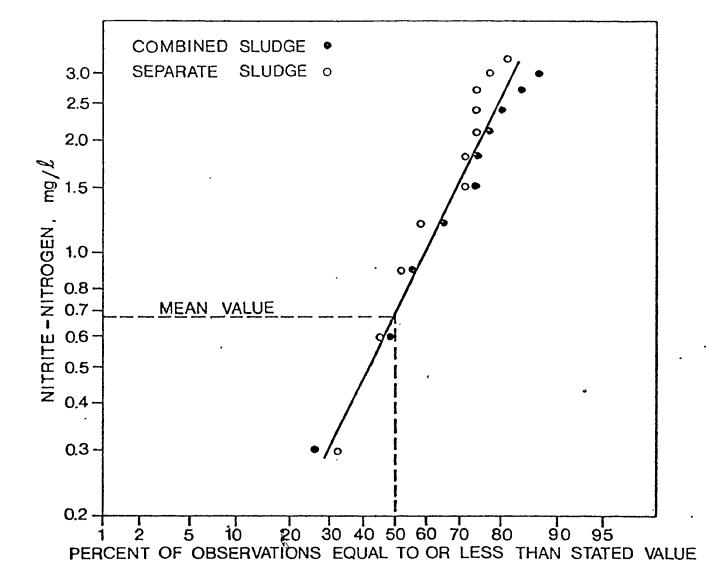
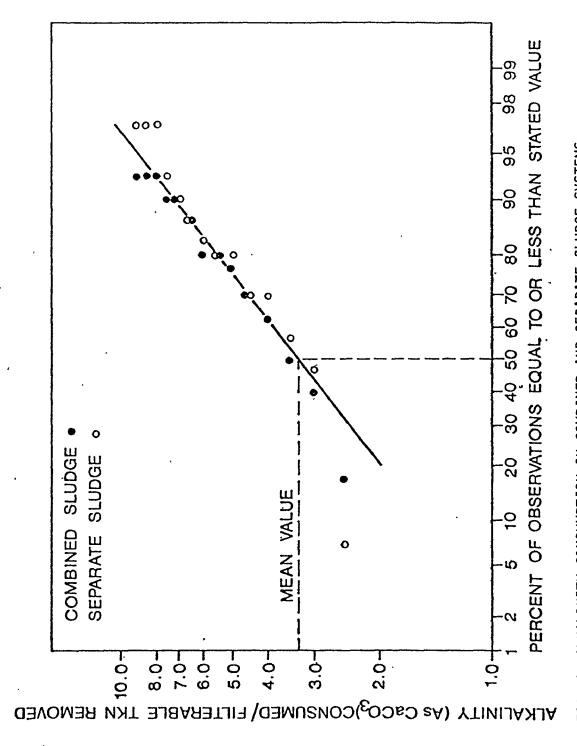


FIG. 8 EFFLUENT NITRITE NITROGEN CONCENTRATION FROM COMBINED AND SEPARATE SLUDGE SYSTEMS



ALKALINITY CONSUMPTION IN COMBINED AND SEPARATE SLUDGE SYSTEMS σ FIG.

organisms and that the decrease in filterable TKN is proportional to the decrease in  $NH_4^+$ -N. Therefore, a more appropriate expression for the alkalinity consumption is in terms of the nitrate plus nitrate nitrogen formed. Using the arithmetic mean values from Figures 7 and 8 the consumption ratio is equal to 4.9 grams of alkalinity (as  $CaCO_3$ ) per gram of  $NO_3^- + NO_2^-$ N formed. This value is still less than the theoretical value which is calculated from equations 6 and 7 to be 7.2. Wilson (1975) and Mulburger (1971) working with supported and suspended growth nitrifying systems respectively, found values of 5.9 and 6.1 grams of alkalinity consumed (as  $CaCO_3$ ) per gram of oxidized nitrogen formed.

A paired "t-test" to determine whether there was a difference in suspended solids in the final clarifier effluents from the combined and separate sludge systems indicated a significant difference at the 95% but not at the 99% level (Table 8, TSS vs TSC). No statistical difference was indicated in comparing the two combined sludge systems (Table 9). The mean clarifier effluent suspended solids from both separate and combined sludge systems was 29 mg/l (Figure 10).

In certain instances a nitrogen control program may necessitate the addition of nitrification facilities without subsequent denitrification. In such cases, it is likely that a TKN limit will be specified. Even with complete nitrification, the clarified effluent can be expected to contain a small quantity of filterable organic nitrogen (Figure 11) probably associated with refractory compounds or metabolic by-products. The analytical determination of this material does not appear to be interfered with by the presence of  $NO_3^-N$  concentrations (Appendix C, Figure C2). In addition, the clarified effluent will contain 1.0 mg/l non-filterable TKN (Figure 12) caused by the presence of suspended solids as indicated in Figure 10.

# Temperature and SRT effects on nitrification

In assessing the performance of the combined and separate sludge systems the analysis of paired data gives no indication of the effect on nitrification of the factors SRT and temperature. The results

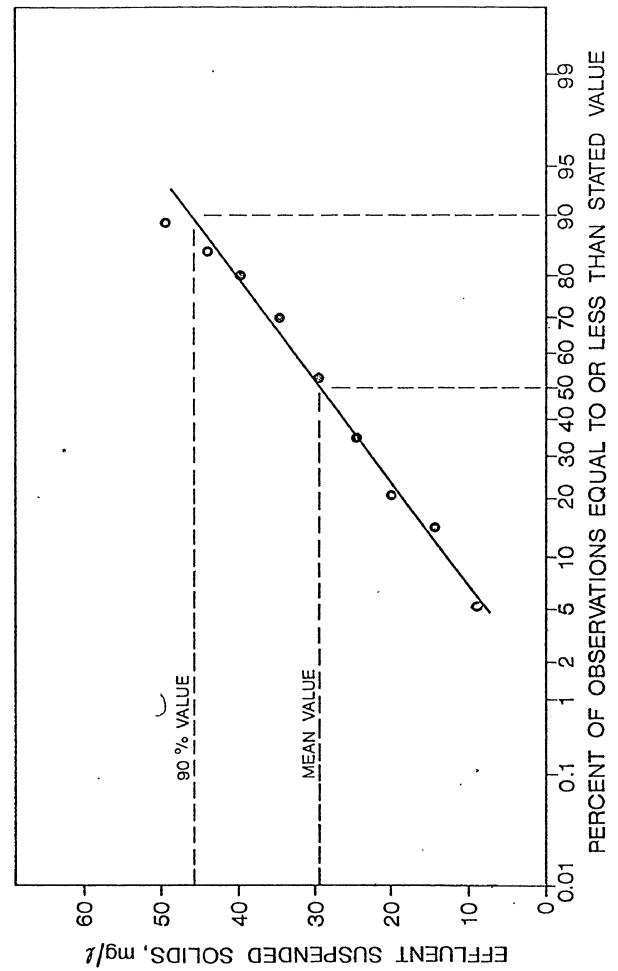


FIG. 10 CLARIFIER EFFLUENT SUSPENDED SOLIDS FROM CARBON REMOVAL-NITRIFICATION SYSTEMS

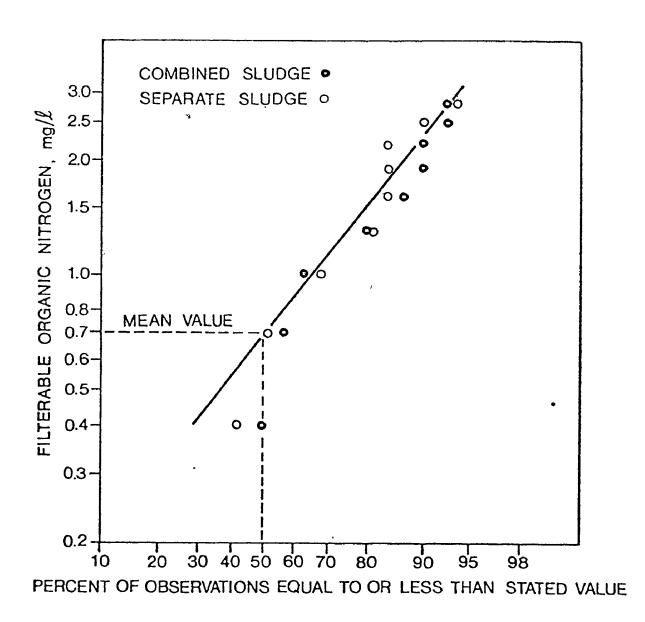


FIG. 11 FILTERABLE ORGANIC NITROGEN FROM CARBON REMOVAL-NITRIFICATION SYSTEMS

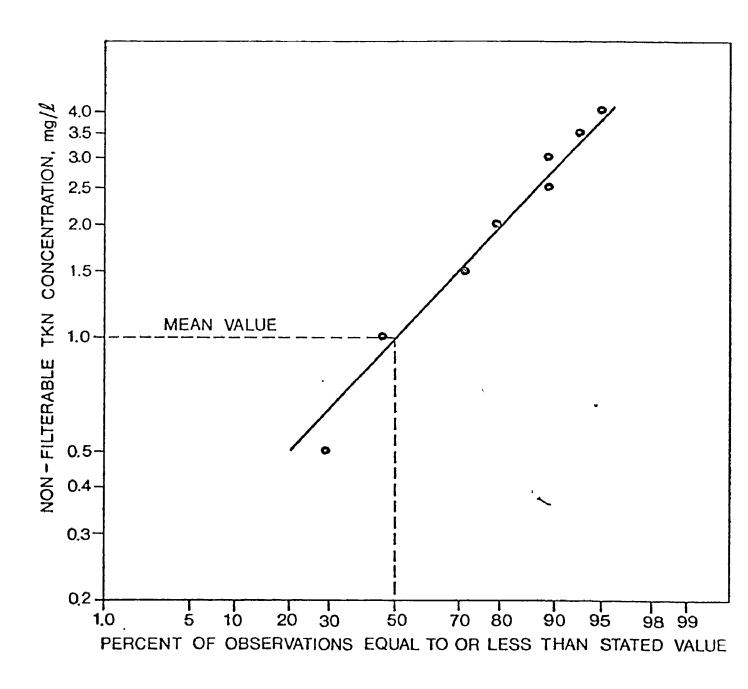


FIG. 12 NON-FILTERABLE EFFLUENT TKN FROM CARBON REMOVAL-NITRIFICATION SYSTEMS

of experiments, designed to compare the alternatives (Table 4), can be used to determine the importance of these factors through an analysis of variance (ANOVA). Tabulation of the TSS sludge system experiments in a form more suitable for an ANOVA (Table 10) reveals a complete factorial design, replicated once, at three levels of SRT and five levels of temperature (3x5). This same design (Table 10) applies to the combined sludge system when the SSC and TSC experiments are considered as one data set.

Temperature and SRT significantly affect filterable TKN removal in combined and separate sludge systems indicated by the values of the variance ratio being greater than the critical "F" value (ANOVA, Table 11). No significant temperature-SRT interaction is evident (Table 11). When the relation between the response (filterable TKN removal) and the levels of SRT and temperature are considered a more detailed ANOVA can be constructed. Since the levels of each variable are at equal intervals (Table 3) the components (linear, quadratic, cubic, etc.) of the main effects of the variables and of the interaction can be determined by means of orthogonal polynomials. This procedure may not only reveal significant main effects and interactions depending on the variable levels, which may have been masked in the overall mean square determination (Table 11), but can give an indication of the relationship between the response and the variables. Applying this procedure, a positive linear correlation is indicated between SRT and filterable TKN removal in the combined and separate sludge systems (Appendix E, Table El). A curvilinear relationship may be more appropriate to describe the positive relationship of temperature and filterable TKN removal (Table El). A summary of the calculations involved in preparing Table El are given in Appendix E.

The relationships between filterable TKN removal and SRT and temperature can be utilized in the design of carbon removal-nitrification systems. Another design approach for nitrification systems is the use of the unit rate of filterable TKN removal. The "zero-order" nature of the nitrification reaction allows the unit rate to be expressed as a constant (K). The ANOVA based on the experiments designed to compare the alternative carbon removal - nitrification alternatives (Table 10) can be constructed

TABLE 10 TEMPERATURE-SRT FACTORIAL DESIGN

Level of SRT		Leve	l of Tempe	erature	
	1	2	3	4	5
-	PSS-07	PSS-58	PSS-29	PSS-44	PSS-61
	PSS-08	PSS-11	PSS-12	PSS-49	PSS-62
0	PSS-01	PSS-20	PSS-25	PSS-31	PSS-53
	PSS-17	PSS-21	PSS-23	PSS-34	PSS-52
+	PSS-14	PSS-59	PSS-13	PSS-16	PSS-55
	PSS-15	PSS-57	PSS-60	PSS-63	PSS-54

EFFECTS OF SYSTEM SRT AND TEMPERATURE ON FILTERABLE TKN REMOVAL IN SEPARATE AND COMBINED SLUDGE SYSTEMS TABLE 11

Main Effects SRT: TSS SSC or TSC 886.20	,	(WS)	Ratio (MS/MSPE)	α=.95
	22	467.82 443.10	14.49 15.10	3.68
Temp.: TSS 7339.02 SSC or TSC 10091.20	44	1834.75 2552.80	56.84 85.96	3.06
SRT and Temp, Interaction: TSS 364.97 SSC or TSC 442.01	<b>∞</b> ∞	45.62 55.25	1.41	2.64 2.64
Pure Error (PE): TSS 484.23 SSC or TSC 440.15	15	32.28 29.34	t I	1 1

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with the unit rate of filterable TKN removal as the response. The unit rate values for these experiments together with the results of other pseudo "steady-state" experiments undertaken to allow for complete system design are tabulated in Appendix C (Table C1).

The unit rate of filterable TKN removal in combined and separate sludge systems is significantly affected by temperature (Table 12). The system SRT has an important effect on the unit rate in the combined sludge systems. In the separate sludge system the result is dependent on the level of the variable as indicated by the more detailed ANOVA (Appendix E, Table E2). In both combined and separate sludge systems there are significant temperature - SRT interactions (Tables 12 and E2) affecting the unit filterable TKN removal rate.

In order to be useful for system design it is important to express the effects of temperature and SRT on nitrification in a concise fashion. The Arrhenius relationship (equation 16) has often been used to represent the variation of substrate removal with temperature in biological systems. A reparamaterized form of this relationship is:

$$K = K^* e^{-E/R(\frac{1}{T} - \frac{1}{To})},$$
 .....(22)

where  $K^* = Ae^{-E/RTO}$ ,

K = reaction rate constant (day<sup>-1</sup>),

A = frequency factor (day<sup>-1</sup>),

 $E = activation energy (cal q-mole^{-1}),$ 

R = universal gas constant (cal g-mole<sup>-1</sup>  ${}^{0}K^{-1}$ ).

T = temperature  $({}^{0}K)$ , and

To = median of the temperature range  $({}^{\circ}K)$ .

This form minimizes the interaction between the frequency factor (A) and the activation energy (E) which makes the Arrhenius equation a difficult expression to fit.

An analysis of variance (ANOVA) indicated no lack of fit, at a critical "F" value of  $\alpha$  = .99, when the reparameterized Arrhenius model was applied to the separate or combined sludge system unit rate data at each individual system SRT of 4, 7, and 10 days (Table 13) based on the complete pseudo "steady-state" data (Table C1). The details concerning the

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EFFECTS OF SYSTEM SRT AND TEMPERATURE ON THE UNIT RATE OF FILTERABLE TKN REMOVAL IN SEPARATE AND COMBINED SLUDGE SYSTEMS TABLE 12

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Source of Variation	*Sum of Squares	Degrees of Freedom	*Mean Square (MS)	Variance Ratio (MS/MSPE)	F α=.95
Main Effects		,			
SRT: TSS SSC or TSC	2.64	22	1.32	2.75	3.68 3.68
Temp.: TSS SSC or TSC	125.98 18.84	44	31.50	65.61 78.50	3.06
SRT and Temp. Interaction: TSS SSC or TSC	13.50	ထထ	1.69	3.52	2.64
Pure Error (PE): TSS SSC or TSC	7.18 0.86	15 25	0.48	1 !	1 1
Note: *values x 10 <sup>3</sup>	•				

TABLE 13 UNIT NITRIFICATION RATE VARIATION WITH TEMPERATURE

Reactor Configuration	Temp. Range	Arrhenius Model E cal/g-mole	Model Parameters A*	Anova Results MSLOF/MSPE	α = .95	F α≅.99
Carbon Removal -Nitrification Systems						
Combined Sludge 4 day SRT		24700	1.47×1017	3.73	3.25	5.41
7 day SRT 10 day SRT	5-25 5-25	13300 12450	3.06×10° 8.29×10 <sup>7</sup>	3.57 0.45	2.64 5.19	4.00
(pooled data with K/K*)				31.08	1.9	2.5
Separate Sludge , 4 day SRT		29700	$1.45 \times 10^{21}$	5.59	3,33	5.64
7 day SRT 10 day SRT	5-25	17550 16950	9.76×10 <sup>11</sup> 4.66×10 <sup>11</sup>	1.86 0.25	2.70	4.14
(pooled data with K/K*)				35.52	2.0	2.5
Nitrification System 6 day SRI	6-25	32750	4.68x1023	1.69	3.33	5.64
10 day SRT 15 day SRT	6-25 6-25	20750 188 <b>50</b>	3.64×10!4 1.48×10 <sup>13</sup>	4.16 0.33	2.70 5.19	4.14
*Note: In calculating A r	reference temp.	temp. = To	$= 273 + 15 = 288^{0}$ K	ە بى		



determination of the ANOVA results are given in Appendix E. The Arrhenius models are illustrated in Figures 13, 14 and 15 together with the models for the nitrification reactor (B2) of the separate sludge system. A system solids retention time of four, seven and ten days in the separate sludge system corresponds to values of six, ten, and fifteen days in the nitrification reactor. To examine any difference in temperature sensitivity between the four, seven, and ten day SRT's for the combined and separate sludge systems, the data were pooled using the ratio of K/K\* in equation 22. The lack of fit of the resulting single models (Table 13) indicates a significant temperature - SRT interaction verifying the previous results (Table 12 and E2). The decrease in activation energy (E) with increased SRT indicates reduced temperature sensitivity at high SRT values. The observed decrease in activation energy is caused by different relative changes in the fraction of nitrifiers present for a given temperature change for systems at different sludge ages. The importance of defining the system SRT in stating nitrification rates is illustrated by the variation in the Arrhenius parameter values (Table 13). The difference in rates observed by other authors (Figure 16) may be due to differences in SRT and/or the influent  $\mathrm{BOD}_5/\mathrm{TKN}$  ratio, another important variable shown to effect the fraction of nitrifiers in an activated sludge plant (Brown and Caldwell, 1975).

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The relationship between SRT and the growth rate of organisms in an activated sludge plant (equation 9) presents a more fundamental approach to design for nitrification in a combined sludge system. This relationship is indirectly represented according to the correlation between the amount of filterable TKN removal and SRT and temperature. Expressing the effect of temperature on the amount of filterable TKN removal, according to the modified Arrhenius relationship (equation 22), the resulting individual models for the combined and separate sludge systems together with the models for the nitrification reactor (B2) are illustrated in Figures 17, 18, and 19. In this case the constant K represents the removal of filterable TKN.

Pooling the filterable TKN removal data to examine any difference in temperature sensitivity, an ANOVA indicated that the resulting single model for the combined sludge system was statistically unacceptable (Table E5).

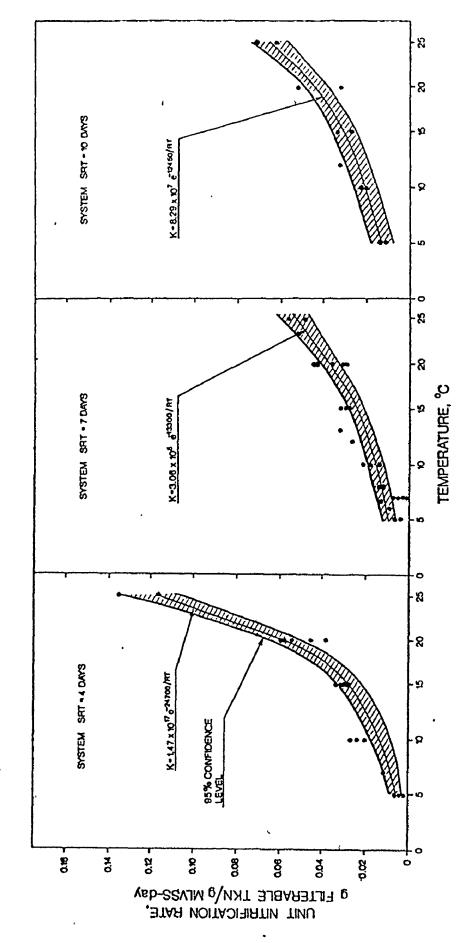
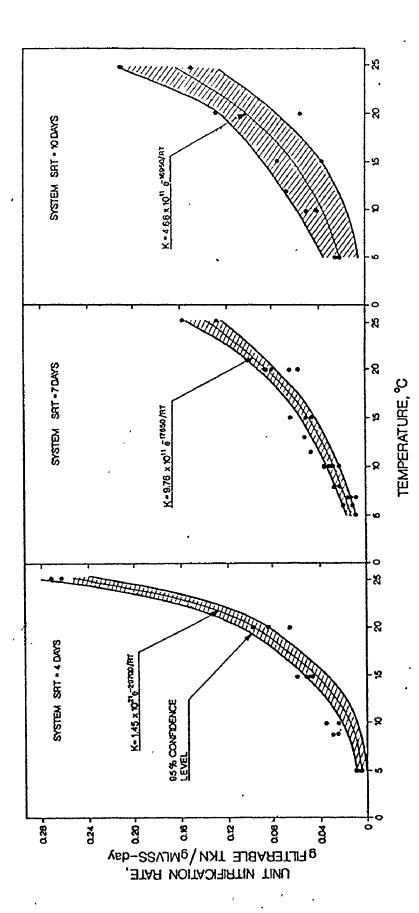
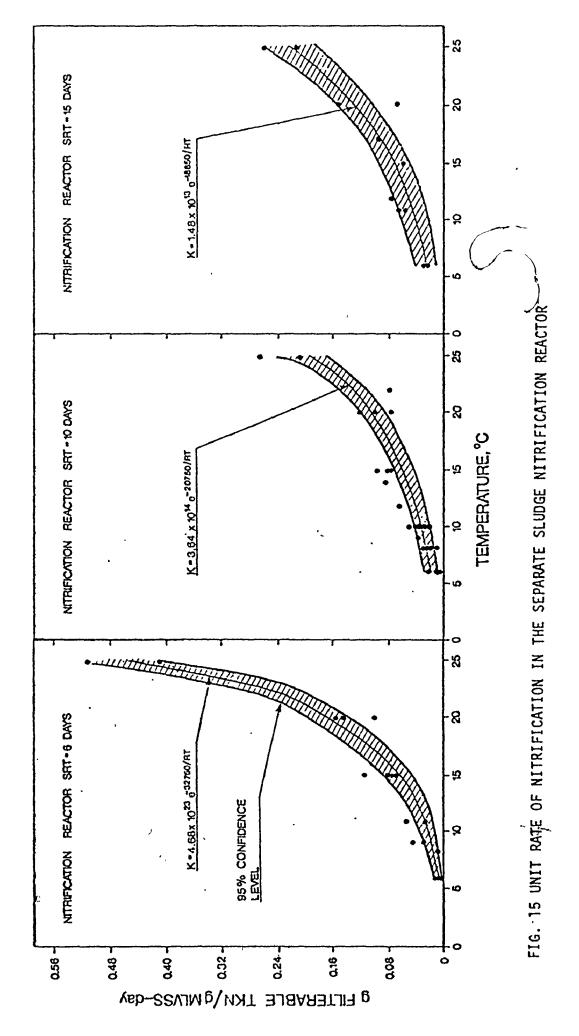


FIG. 13 UNIT RATE OF NITRIFICATION IN THE COMBINED SLUDGE SYSTEM



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UNIT RATE OF NITRIFICATION IN THE SEPARATE SLUDGE SYSTEM FIG. 14



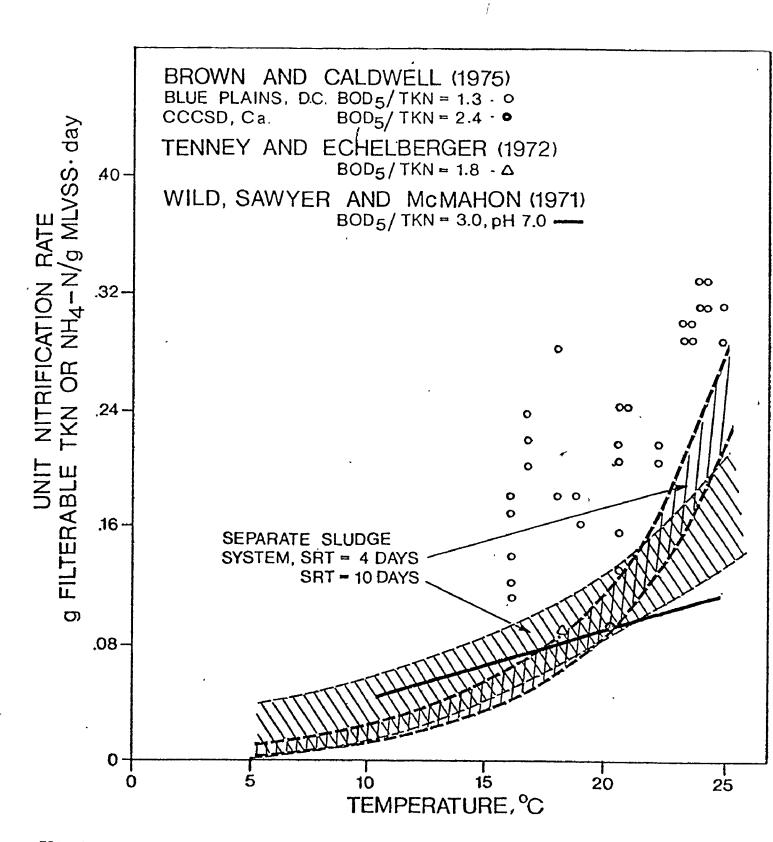
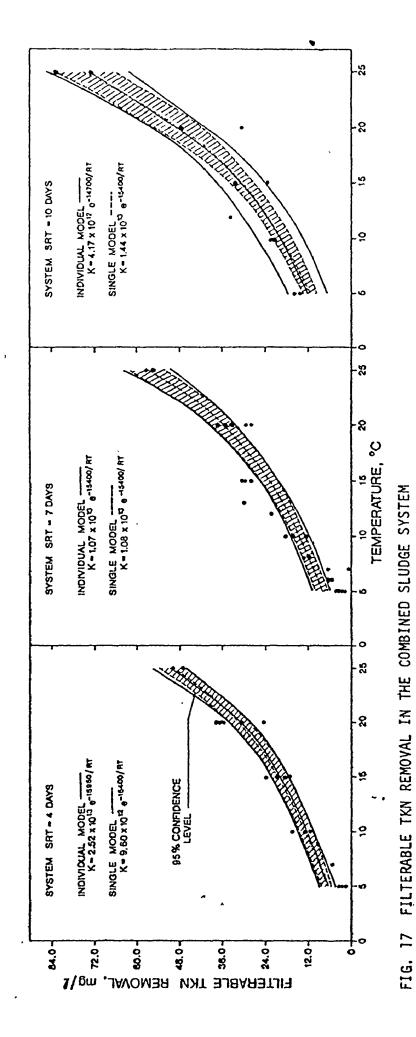
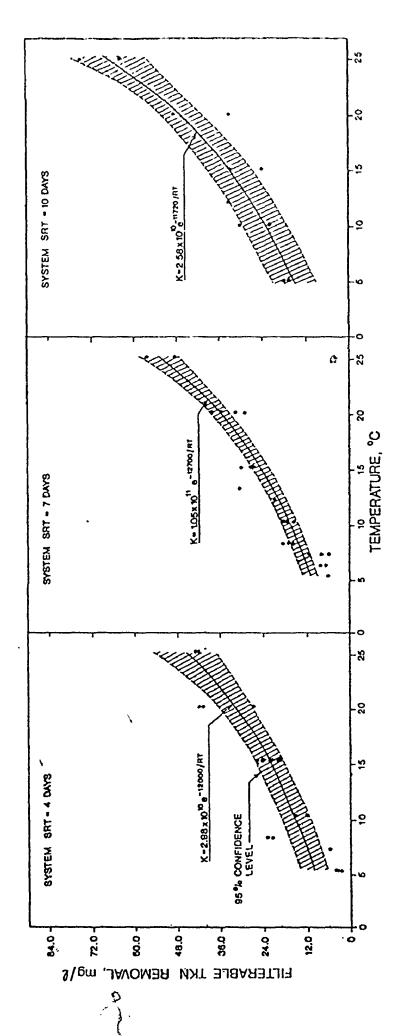


FIG. 16 NITRIFICATION RATES REPORTED BY VARIOUS AUTHORS





FILTERABLE TKN REMOVAL IN THE SEPARATE SLUDGE SYSTEM

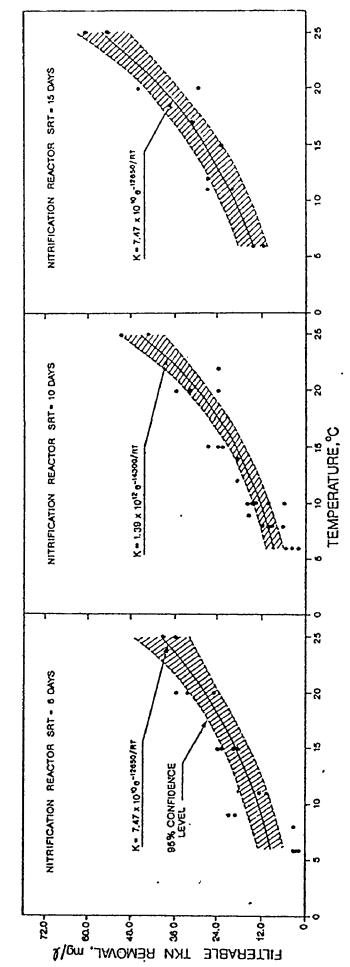


FIG. 19 FILTERABLE TKN REMOVAL IN THE SEPARATE SLUDGE NITRIFICATION REACTOR

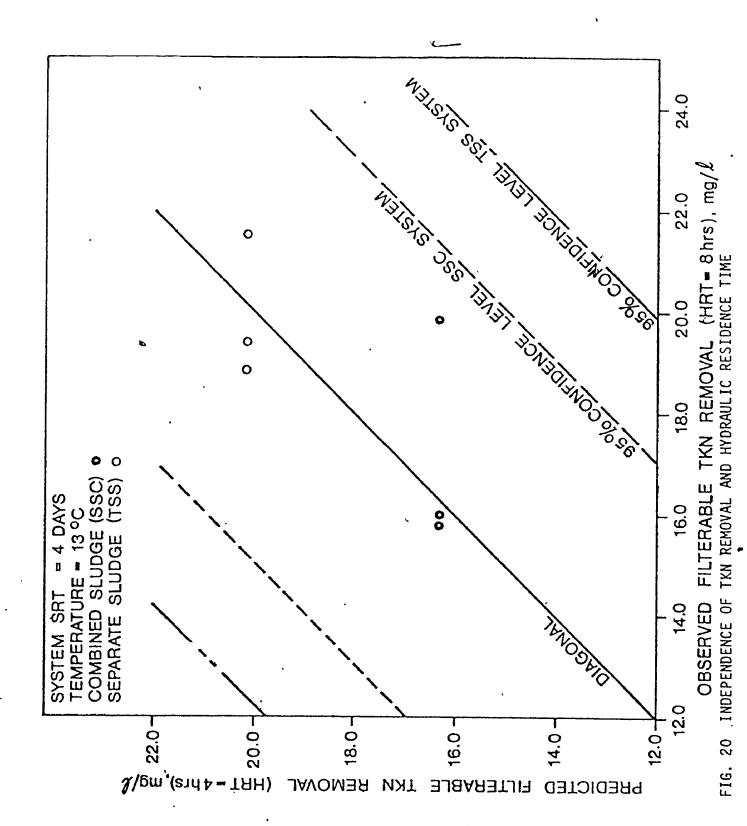
A similar result was found for the separate sludge system. The difference between the single models with a common activation energy (E), and the individual models is not readily discernible (Figure 17). This observation is supported by the lack of a temperature - SRT interaction previously noted (Table 11) in analyzing the results of the experiments designed to compare the alternative systems. A common temperature sensitive reflects equal relative changes in the number of nitrifiers present for a given temperature change for systems at different sludge ages.

The filterable TKN removal - SRT - temperature relationships (Figures 17, 18 and 19) suggest an independence of TKN removal and hydraulic residence time (HRT). This is verified by examining observed results at a substantially different HRT but at equal SRT and temperature conditions (Figure 20, Table C4).

By presenting the performance of the combined carbon removalnitrification system according to Figure 17, an independence is implied between TKN removal and the BOD<sub>5</sub> loading. A different value for the influent BOD<sub>5</sub> from that encountered in this study will result in a difference in cell yield (heterotrophs) at the same SRT, and subsequently a difference in the amount of N removed by synthesis. Accounting for this difference, the pseudo "steady-state" design of nitrifying activated sludge system can be specified by noting the SRT necessary for a given filterable TKN removal. Fixing the SRT establishes the growth rate of both the autotrophic nitrifier population and the heterotrophic organisms. From the growth rate-substrate removal rate relationship for the heterotrophic population the other necessary design parameters (hydraulic detention time, mixed-liquor volatile, suspended solids concentration, etc.) can be established (Lawrence and McCarty, 1970).

#### Solids Production

Any postulated advantages of the separate sludge system such as greater stability or increased buffering capacity to compounds toxic or inhibitory to nitrification must be balanced against the additional cost of added clarification facilities and increased solids production. By determining the cumulative solids wasted during a pseudo "steady-state" operating period either in the process effluent or by intentional daily



wasting, solids production from the parallel operating alternatives was assessed. During the study, an average 1.6 kg of solids was wasted from the separate sludge system for every 1 kg wasted from the combined system (Figure 21).

### Dynamic Behaviour of Nitrification Systems

The results of the experiments performed under non-steady conditions (Table 5) are included in Appendix B.

#### System response to pH and temperature changes

The transitional or short term response of the combined (SSC) and separate (TSS) sludge systems to changes in temperature and pH was investigated first. Parallel operation afforded a direct qualitative comparison of the response and recovery profiles of the carbon removal-nitrification systems.

A slower response to a step-down in temperature is indicated (Figure 22) for the separate sludge system. The approach by this sytem to TKN values "predicted" by the unit rate models at system SRT's of four days (Figures 13 and 14), lagged behind the combined sludge system. The minimum attainable filterable TKN concentration from the combined and separate sludge systems is 0.7 mg/l (Figure 9). The change in effluent filterable COD concentration further indicated the stability of the separate sludge system to changes in temperature.

The response to a reduction in feed pH was a significant change in effluent pH, and filterable TKN concentration (Figure 23). The separate systems exhibited a buffering capacity to a change in system pH which is reflected by the lag in response and recovery in terms of filterable effluent TKN concentration. This might be explained by the difference in system hydraulics. The nitrifying reactor (B2), of the separate sludge system (B1 and B2), exhibited a pH profile having a greater magnitude and a shorter duration than that of the combined sludge reactor. The "predicted" increased effluent filterable TKN reflects the loss in reactor volatile solids caused by decreased settleability. The observed effluent filterable COD concentrations

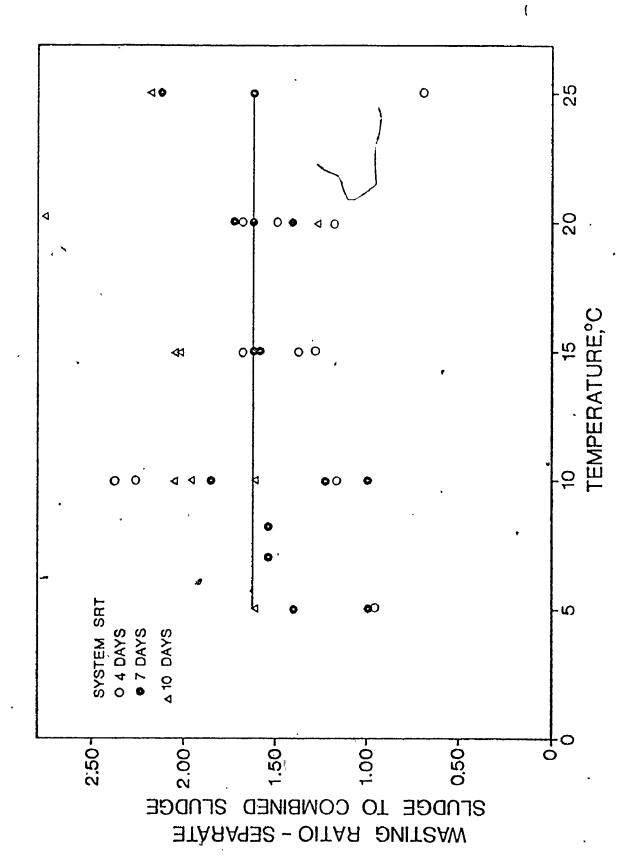


FIG. 21 REQUIRED SOLIDS WASTING IN SEPARATE AND COMBINED SLUDGE SYSTEMS

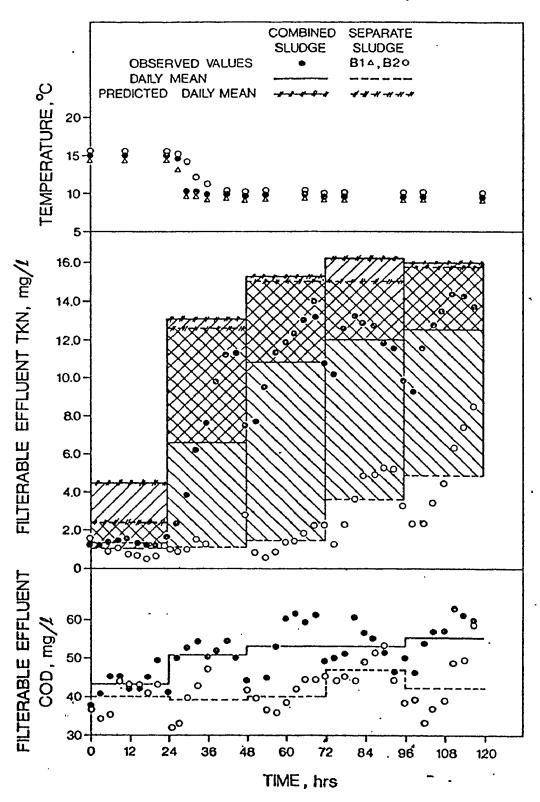


FIG. 22 RESPONSE OF CARBON REMOVAL NITRIFICATION SYSTEMS TO TEMPERATURE STEP-DOWN

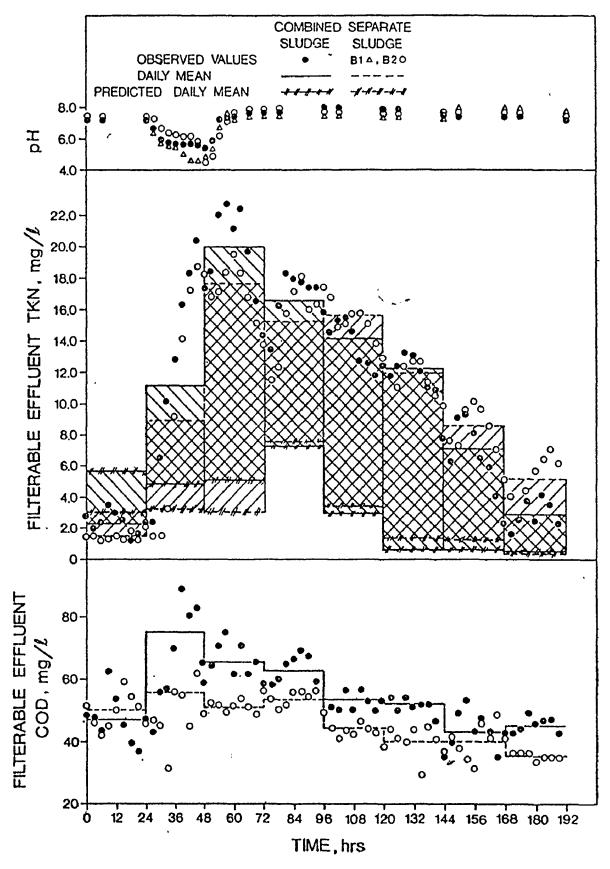


FIG. 23 RESPONSE OF CARBON REMOVAL-NITRIFICATION SYSTEMS TO PH IMPULSE

(Figure 23), indicate the reduced effect of the toxic conditions on the separate sludge system.

## System response to hydraulic, organic, and inorganic loading variations

Deterministic and linear dynamic - stochastic models were both considered in attempting to describe the response of the carbon removal-nitrification systems to variations in hydraulic, organic, and inorganic loading. The complexity of the activated sludge process, operated for carbon removal-nitrification, made it necessary to make a number of simplifying assumptions in the development of a deterministic model for the SSC sludge system. Even with these inherent weaknesses, which will give rise to unexplainable output variations or noise, the usefulness of the model is complicated by the number of time-dependent growth and substrate removal functions (Figure 4). With this in mind transfer function models together with linear time series models were used to describe the response of the combined and separate sludge systems to input variations.

Experiment D5 (Table 5) was designed to allow development of the linear dynamic-stochastic models. The designed input series (Figure 24) separated out the correlation between the input variables allowing assessment of the effects of flow, filterable OC, and filterable TKN on the output concentrations of filterable TKN and nitrate for the two-stage separate and single-stage combined sludge systems.

The cross correlation results indicated that for both sludge systems, the effluent filterable TKN concentration might be expected to increase in response to increases in influent filterable TKN concentration and flow upon the two hour analytical results (Figure 25). No significant response to changes in influent filterable OC concentration would be expected (Figure 25).

In evaluating the effluent nitrate-nitrogen response, a positive correlation to influent filterable TKN was indicated. The negative trend of the cross correlation functions between effluent nitrate concentration and influent filterable OC and flow indicated a possible significant negative

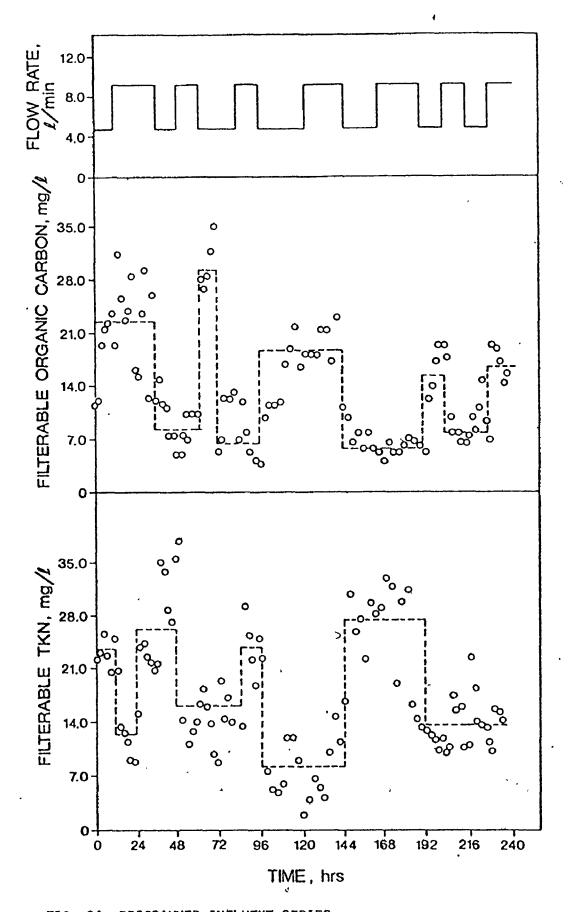


FIG. 24 PROGRAMMED INFLUENT SERIES

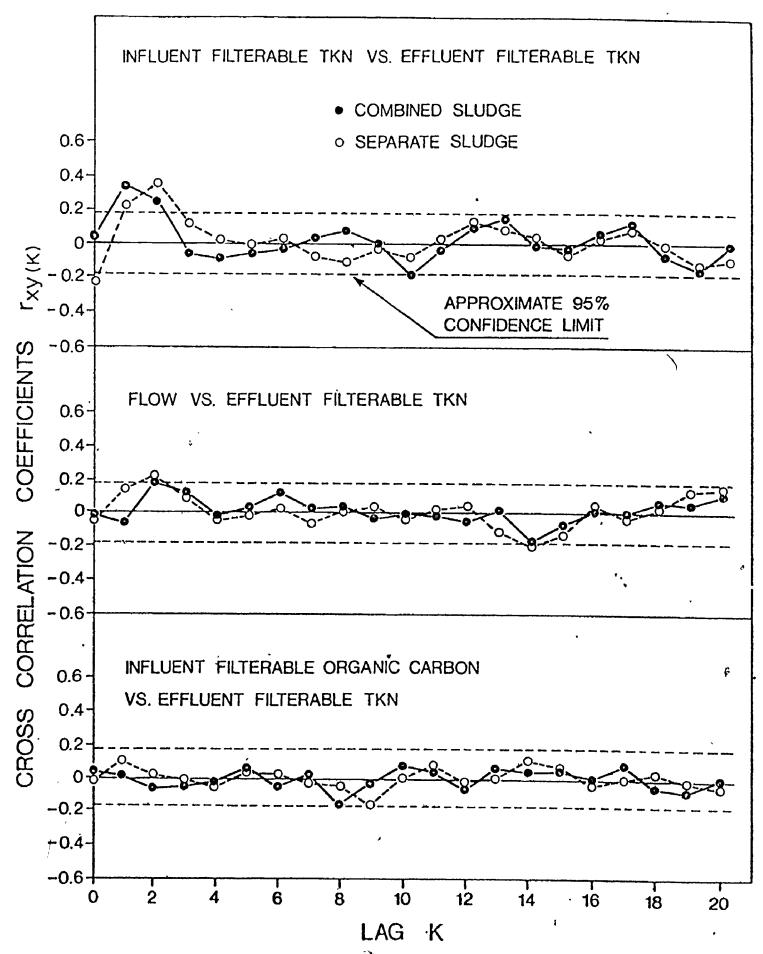


FIG. 25 CROSS CORRELATION RESULTS FOR EFFLUENT FILTERABLE TKN

correlation when the filterable OC loading (filterable OC x flow) was considered as the input variable to the separate and combined sludge systems (Figure 26).

Initial parameter values for the tentative transfer function models relating the output filterable TKN and  $NO_3^-N$  values to the input variables were calculated by first determining the impulse response weights (Vi). The parameters in the tentative models were then estimated using non-linear least squares. Examination of the residuals of the fitted models led to the identification of the noise model orders. Reestimation of the parameters led to combined transfer function noise models for effluent filterable TKN and  $NO_3^-N$  (Tables Fl and F2). Comparison procedures and individual evaluation through diagnostic checking (Appendix F) led to the final model forms (Table 14). The computer programs used to determine auto and cross correlation results, impulse response weights, and least square parameter values are included in Appendix F together with examples of the programming results.

The observed and model results (Figure 27, Table 14) indicate that the effluent filterable TKN concentration from both systems responded positively to variations in TKN loading, but showed no significant response to changes in filterable OC concentration or loading. The magnitude and lag of the response was similar for both systems. The effluent  $NO_3^-N$  concentration from the separate and combined sludge systems responded positively to changes in filterable TKN concentration and negatively to filterable OC loading.

Parallel reactor operation afforded insight into the response and recovery profiles of effluent filterable TKN from the combined and separate sludge systems when they were subjected to sudden changes in flow and flow plus filterable OC and TKN concentration levels (Table 5, D3 and D4). The observed results indicate that the magnitude of the response was greater in the separate sludge system (Figure 28). This may be due to the difference in hydraulic properties of the two systems. The results of experiment D9 (Figure 29) indicate the further the deviation from complete mixing, the greater the magnitude of the response. The value of the dimensionless dispersion parameter (D/uL) for the five-

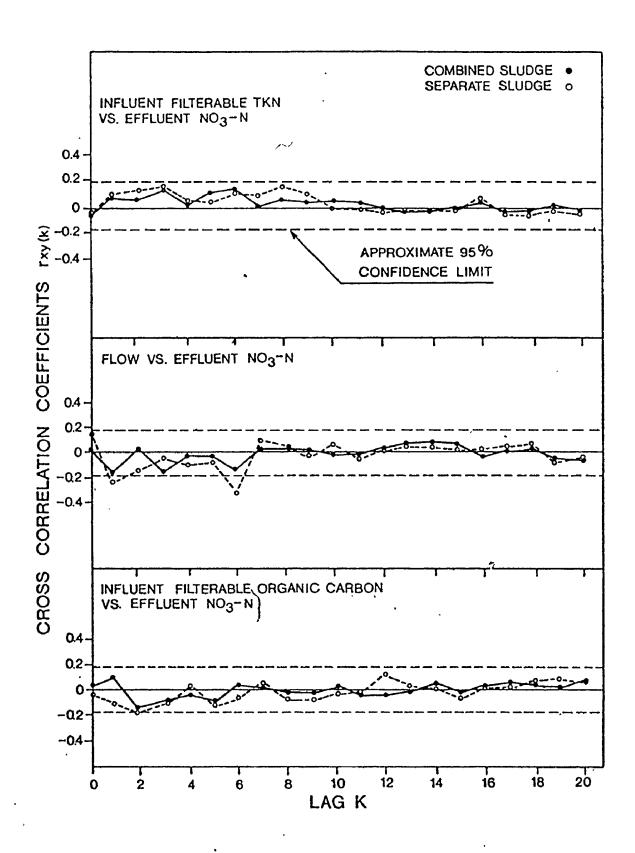


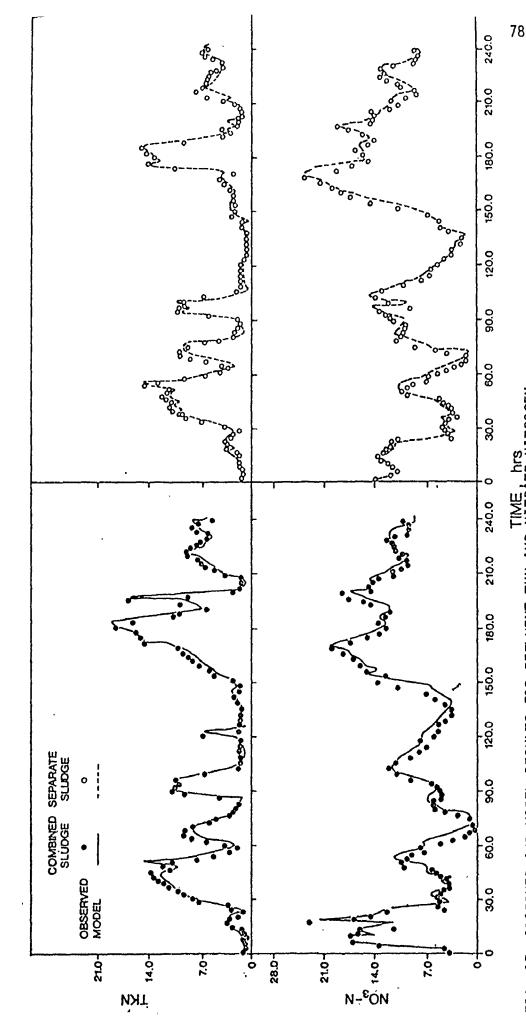
FIG. 26 CROSS CORRELATION RESULTS FOR EFFLUENT NITRATE NITROGEN

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TRANSFER FUNCTION - NOISE MODELS DESCRIBING OBSERVED EFFLUENT TKN AND NITRATE-NITROGEN TABLE 14

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Variable	System
	Combined $Y_t = \frac{0.011}{1-0.786B} X_{1_{t-1}}^{1} + \frac{1}{1-1.066B} + 0.336B^Z$ at
Filterable TKN	Separate $Y_t = \frac{0.012 + 0.0088}{1 - 0.5508} X_{1+1} + \frac{1}{1-1.3948 + 0.5408^2}$ at
	Combined $Y_t = 0.051 \times 2_{t-3} - \frac{0.003}{1-0.7448} \times 3_{t-3} + \frac{1}{1-0.8898}$ at
N03-N	Separate Yt = 0.049 $x_{2_{t-3}} - \frac{0.004}{1-0.8158} x_{3_{t-1}} + \frac{1}{1-1.1278} + \frac{1}{0.2218^2}$ at
where $X_1 = TKN$ $X_2 = Inf$ $X_3 = 0C$	(filtera luent TKN (filterab



TIME, hrs FIG. 27 OBSERVED AND MODEL RESULTS FOR EFFLUENT TKN AND NITRATE NITROGEN

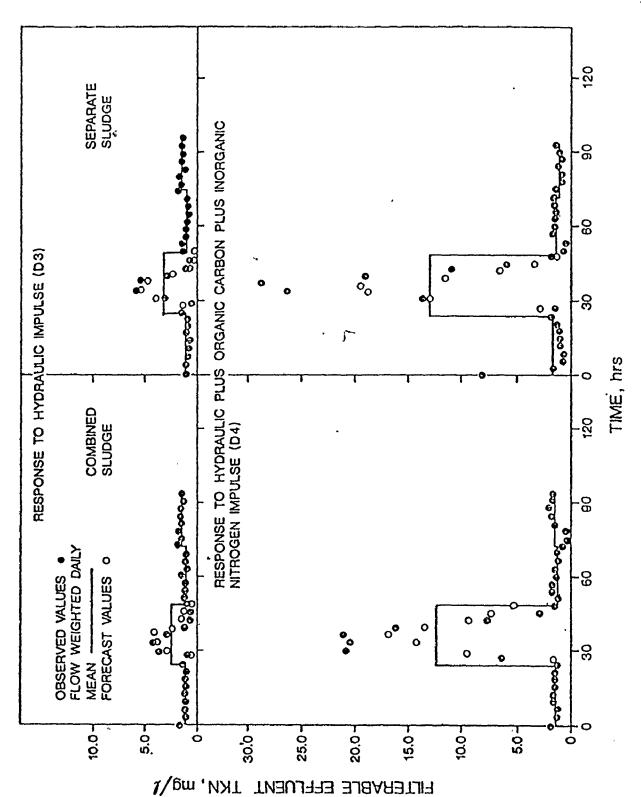


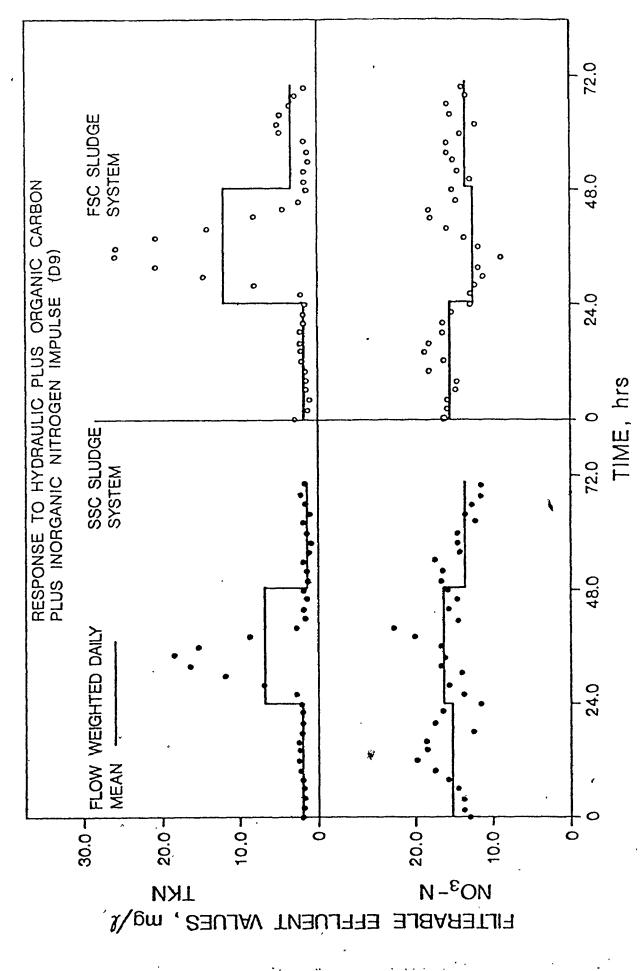
FIG. 28 OBSERVED AND FORECASTED RESPONSE TO IMPULSE LOADINGS

stage combined sludge system (FSC) was found to be less than 0.2 (Appendix D). The  $NO_3^-N$  results (Figure 29) verify that a shock loading condition has a greater impact on a nitrifying system with a mixing regime approaching plug flow. This same observation has been reported for full-scale complete mix and plug flow systems operated in parallel for BOD removal (Toerber, Paulson, and Smith, 1974).

To determine how appropriate the discrete transfer function noise models developed from experiment D5 are as a predictive tool, the effluent filterable TKN response during D3 and D4 was forecast. The models developed (Table 14) were based on two hour analytical results and must be altered to account for the sampling interval of three hours used in the non-steady experiments. The revised filterable TKN models (Table 15) were used to forecast the response of the combined (SSC) and separate sludge system (TSS) to the impulse loading conditions imposed during experiments D3 and D4. The response and recovery times showed good agreement (Figure 28). The magnitude of the response for experiment D4 was under-estimated. The impulse loading condition imposed during this experiment was beyond that encountered in the experiment D5 from which the models were developed. This could account for the difference between forecast and observed results. Details concerning the model revision procedures and forecasting methods are included in Appendix F.

The dynamic response of the combined and—separate sludge systems to step changes in flow rate, organic carbon and inorganic nitrogen was investigated in experiments D6, D7 and D8. The step changes were initiated following observation of the performance of the systems for 24 hours under baseline conditions (Table 5).

In response to a doubling in flow rate, both systems increased in effluent filterable TKN corresponding to a decrease in removal (Figure 30). The pseudo "steady-state" relationships developed predict an independence of TKN removal and hydraulic retention time providing the solids retention time is maintained. Following an initial response, the combined sludge system performance verifies this relationship as a slightly lower removal level is established corresponding to a lower SSRT at the new pseudo "steady-state" (Figure 30). The response in terms of effluent nitrate



RESPONSE OF COMPLETE MIX AND PLUG FLOW NITRIFYING SYSTEMS TO IMPULSE LOADING CONDITIONS FIG. 29

TABLE 15 TRANSFER FUNCTION - NOISE MODELS FOR THREE HOUR SAMPLING INTERVAL

Variable	System	Model
	Combined	$y_t = \frac{0.0156}{1-0.6978} x_{1_{t-1}} + \frac{1}{1-0.72548} + \frac{1}{0.1958^2} a_t$
Filterable IKN	Separate	$\gamma_{t} = \frac{0.020 + 0.006g}{1 - 0.408g} \times_{1_{t-1}}^{1} + \frac{1}{1 - 1.116g + 0.3964g^{2}} $ at
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where $x_1 = TKN$ (filteral	ilterable) loa	ble) loading (g/day).

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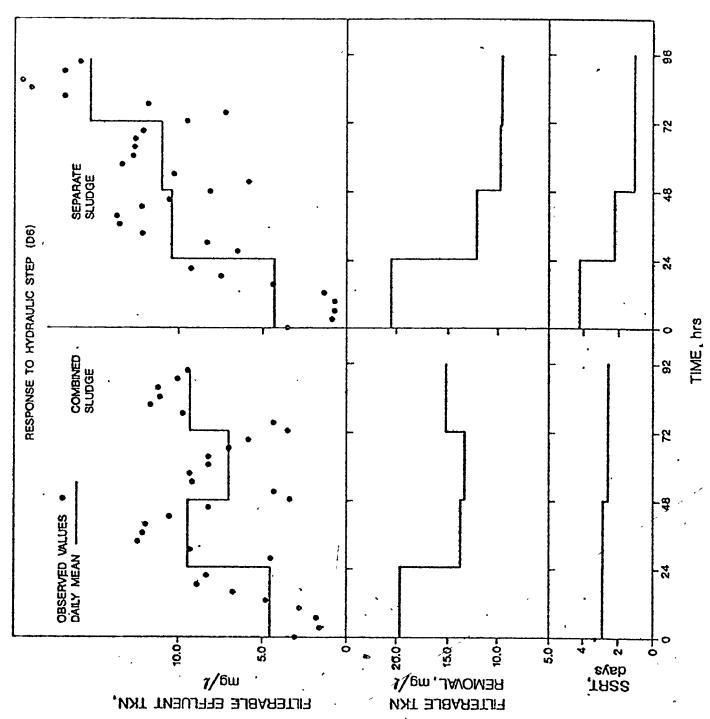
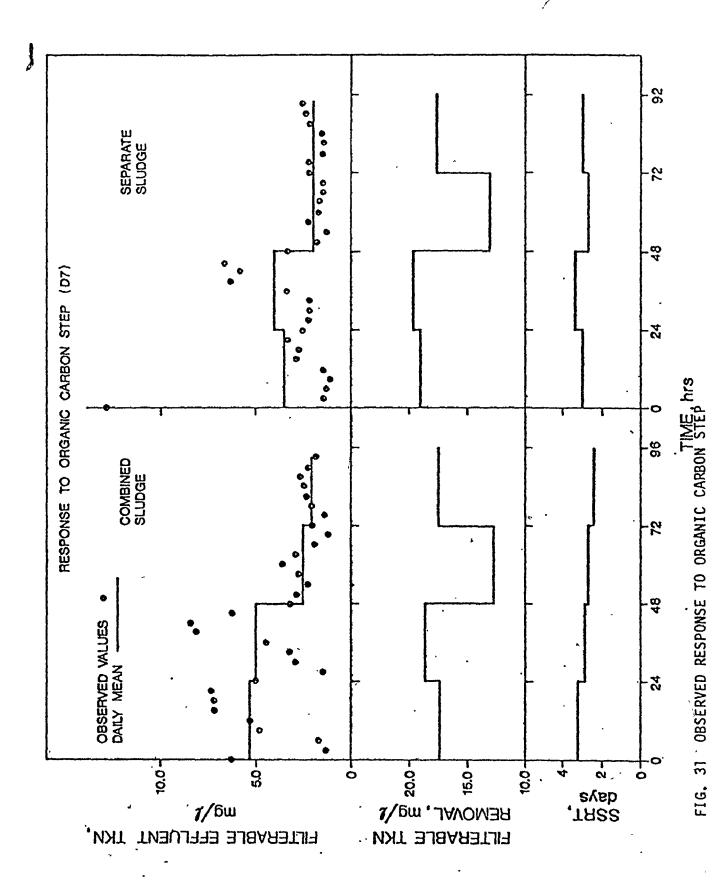


FIG. 30 OBSERVED RESPONSE TO HYDRAULIC STEP

was quite similar verifying the independence of nitrification and HRT (Appendix B). The decrease in filterable TKN removal in the separate sludge system (Figure 30) corresponds to a decrease in SSRT. The decrease in SSRT was caused by the loss of volatile solids in the nitrifying reactor (B2) through increased clarifier effluent suspended solids.

The organic carbon step imposed in D7 resulted in a doubling of the influent filterable BOD. The observed filterable TKN removal indicated an apparent delayed negative response both in the combined and separate sludge systems (Figure 31). This is likely not a true response but is due to a naturally occurring decrease in the influent filterable TKN concentration, limiting the TKN removal as evidenced by the low effluent values (Figure 31). This is supported by the filterable TKN removal results over the last 24 hour period. By maintaining the same system solids residence time (Figure 31), which involved increasing reactor volatile suspended solids together with increasing solids wasting from the reactors, the same number of nitrifiers was maintained and consequently the same filterable TKN removal resulted. This supports the contention that nitrification systems operating under similar environmental conditions (reactor pH, DO, and temperature) will contain the same number of nitrifiers at equal SRT's regardless of the C/N ratio.

In experiment D8 the inorganic nitrogen step imposed on the natural diurnal influent concentration, resulted in an apparent increase in filterable TKN removal (Figure 32) during the first day of addition (24 to 48 hr). For the combined sludge system, the removal results during the following days indicate that this response is likely associated with the system delay. The performance verifies that the combined sludge system was initially operating under critical nitrification conditions after which the same absolute amount of filterable TKN was removed at the stable SSRT conditions. This constant degree of nitrification is verified by equal daily mean effluent NO3-N values during the experiment (Appendix B). The separate sludge system during D8 operated at a significantly higher SSRT due to an experimental error and therefore initially was not



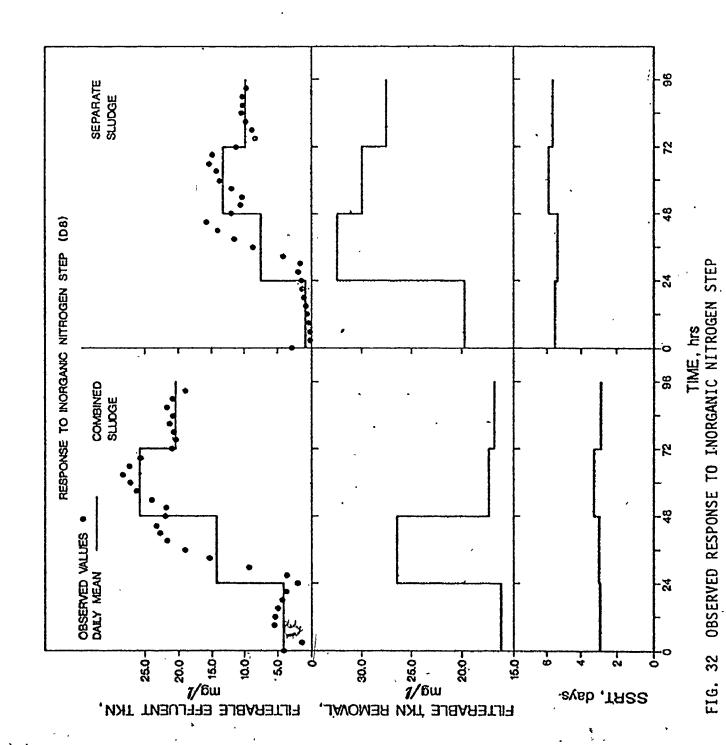
at critical nitrification conditions as evident from the effluent results (Figure 32) during day one (0 to 24 hr). Consequently, an increased degree of filterable TKN removal occurred following the nitrogen step. This result is verified by a corresponding increase in the level of effluent  $NO_3^2$ -N (Appendix B).

# System response to natural diurnal variations in flow and concentration

The step forcings and impulse loading conditions imposed during the non-steady experiments allow us to assess how combined and separate sludge systems operating under critical nitrification conditions respond to sudden changes in influent conditions and changes in the pseudo "steady-state" levels of the input variables. In establishing design criteria for wastewater treatment plants the response to the natural diurnal variation in flow and concentration normally encountered at a wastewater treatment plant is an important consideration. This response can be examined by reference to the baseline day results of the non-steady experiments.

The baseline influent TKN levels during experiments D3 and D6 (Figure 33) represent extremes in the natural concentrations encountered. The resultant TKN loadings lead to a significantly different effluent filterable TKN response (Figure 33) for both the combined and separate sludge systems even though the experimental SSRT (three days) and temperature (13° to 15°C) conditions were similar. The daily mean effluent filterable TKN values during these baseline days (Figures 28 and 30) can be predicted approximately by use of the pseudo "steady-state" TKN removal relationships developed at SSRT's of four days (Figures 17 and T8). In experiment D3 the predicted values for both the combined and separate sludge systems correspond to the residual filterable organic nitrogen concentration (0.7 mg/l) identified previously (Figure 11). In experiment D6 the predicted values are 4.8 and 4.0 mg/l filterable TKN for the combined and separate sludge systems respectively. These predicted values correspond quite closely to the observed daily mean results (Figures 28 and 30) during the baseline days.

In order to reduce the filterable effluent TKN variation in experiment D6 a greater SSRT would be necessary according to the pseudo



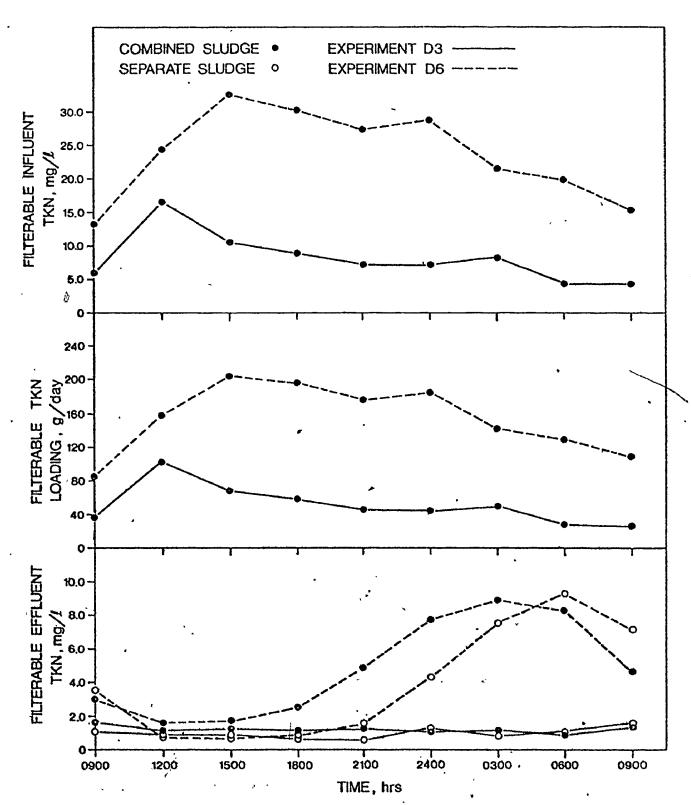


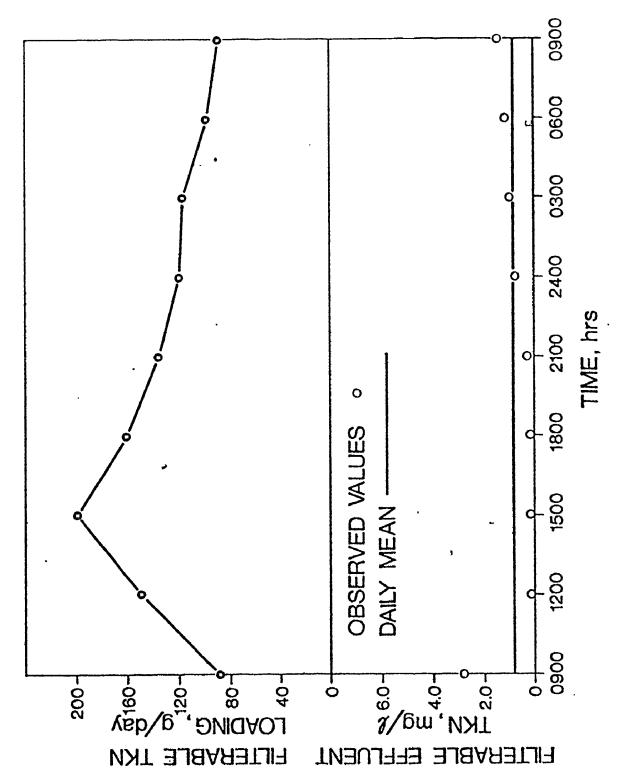
FIG. 33 OBSERVED RESPONSE TO NATURAL DAILY CONCENTRATION VARIATION

"steady-state" relationships developed. This is verified by referenceto the baseline day results for the separate sludge system in experiment D8 (Figure 32). The influent loading conditions were comparable to that encountered in experiment D6 but the effluent filterable TKN variation was reduced to the residual level (Figure 34).

In addition to experimental observation, the dynamic models developed in experiment D5 can be used to examine the response of the combined and separate sludge systems to natural influent concentration variations. The transfer function component of a TF-N model (equation 18) expresses the deterministic nature of the particular process. The noise model accounts for the unexplained output variations. Therefore the effluent variation due to the nature of the combined and separate sludge processes is determined from the transfer function model results.

By simulating the influent conditions encountered during the baseline days of experiments D3 and D6 the filterable TKN effluent variation was predicted using both the transfer function models and the combined transfer function-noise models. The transfer function models predict no significant filterable effluent TKN response in the combined or separate sludge systems for the influent conditions approximated from D3 (Figure 35). These results support the observed values (Figure 33). Simulating the influent conditions encountered in experiment D6, both the transfer function and combined TF-N models predict a significant filterable effluent TKN response for both SSC and TSS sludge systems (Figure 35). The observed effluent variation (Figure 33) over the twenty-four hour period is better predicted by the TF-N models. Details concerning the simulation and prediction methods are included in Appendix F.

The TF-N models were used to examine the response of the carbon removal nitrification systems to natural concentration and flow variation. In simulating hydraulic input conditions, the flow pattern and extent of variation chosen (maximum to average to minimum equal to 1.6:1.0:0.5) were representative of that to the Burlington Skyway Treatment Plant. The mean daily flow was set at the value maintained during the baseline days of the dynamic experiments (4.55 1/min). The simulated level and variation



RESPONSE OF SEPARATE SLUDGE SYSTEM TO D8 BASELINE DAY INPUT FIG. 34

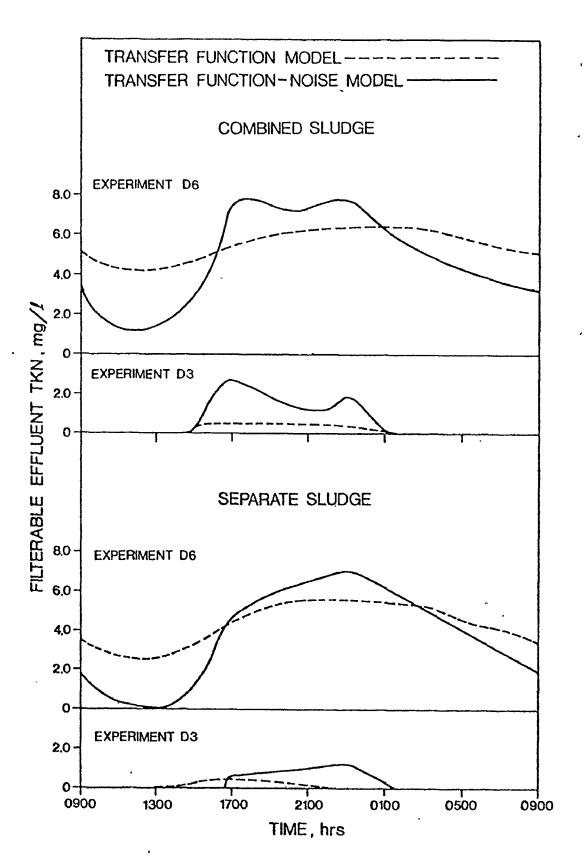


FIG. 35 PREDICTED RESPONSE TO SIMULATED DAILY CONCENTRATION VARIATION

in filterable TKN concentrations was that representing the D6 conditions. In response to these superimposed loading conditions, the TF-N model for the combined sludge system (SSC) predicts a greater effluent variation than for the variable concentration input alone (Figure 36). A similar result was found for the separate sludge system. Although a greater hourly variation in filterable TKN concentration is predicted, little difference exists in the calculated daily mean results (Figure 36) based on flow-weighted sampling. This would indicate that the relationships previously developed representing the daily mean filterable TKN removal performance under pseudo "steady-state" conditions may be used to determine results under variable flow conditions. This contention is supported by the observed results during a screening experiment in Phase 3 (Jan. 1975). In this experiment a sinuoidal flow pattern was superimposed upon the natural concentration variation (Figure 37). The observed response of the combined sludge system (Figure 37) was predicted closely by the pseudo "steady-state" relationship (Figure 17).

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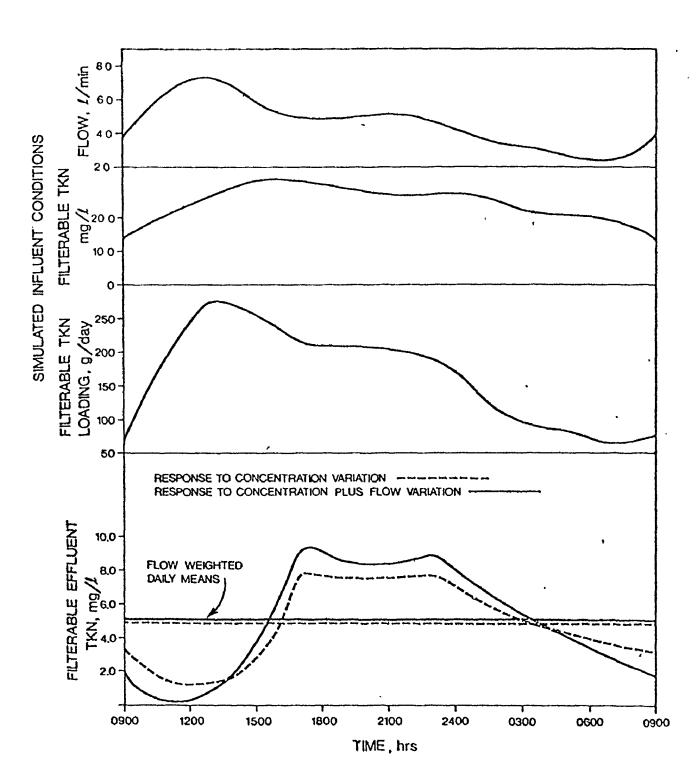


FIG. 36 PREDICTED RESPONSE TO SIMULATED DAILY CONCENTRATION AND FLOW VARIATION

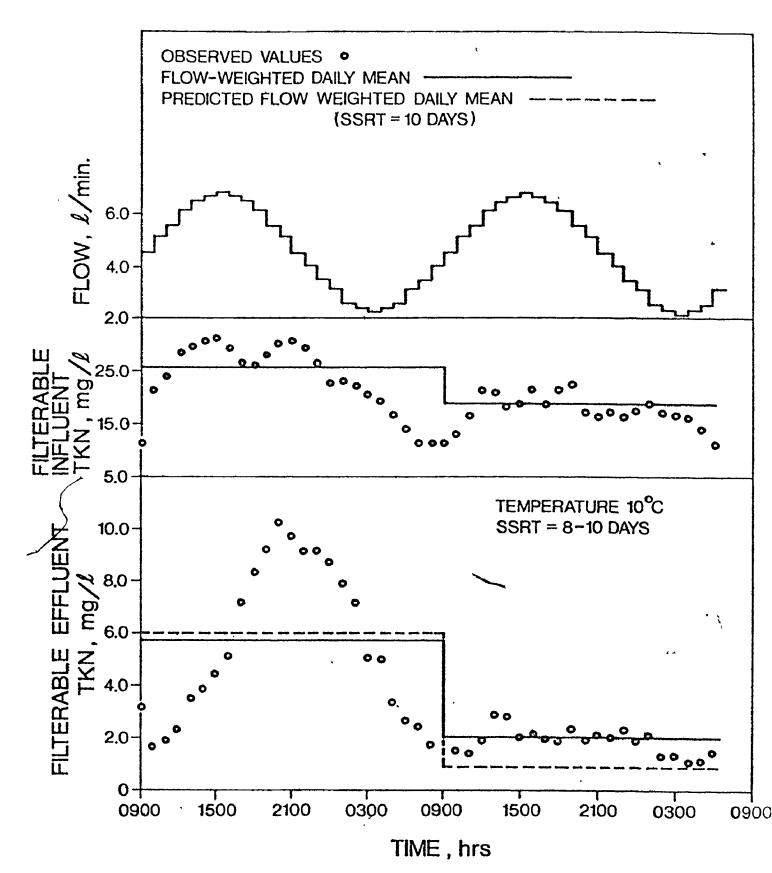


FIG. 37 RESPONSE OF COMBINED SLUDGE SYSTEM TO DAILY CONCENTRATION AND FLOW VARIATION

#### CONCLUSIONS

This study, conducted under pseudo "steady-state" and non-steady conditions, supports the hypothesis that biological nitrification is feasible employing combined or separate sludge systems even under cold climatic conditions.

It may be concluded that under pseudo "steady-state" conditions:

- 1) Equal degrees of nitrification can be accomplished in combined or separate sludge nitrification systems.
- 2) For nitrogen levels commonly found in domestic wastewater, the rate of nitrification, expressed as the filterable TKN removed per unit mass of activated sludge, is independent of the concentration of filterable TKN down to values greater than 1.0 mg/l.
- 3) Even with complete nitrification an average value of approximately 1.0 mg/l of organic nitrogen can be expected in the filterable fraction of the effluent.
- 4) Temperature and solids residence time significantly effect filterable TKN removal in combined and separate sludge systems. Nitrification is essentially independent of hydraulic residence time.
- 5) A separate sludge system will produce a significantly greater amount of sludge compared to a combined sludge system.

An evaluation of the response of combined and separate sludge systems to non-steady influent conditions indicates that:

- A pulse change in influent pH will cause a lower reactor pH but of shorter duration in a separate sludge system. The accompanying increase in effluent concentration of both filterable TKN and COD will be less.
- 2) For a step down in temperature, the increase in effluent filterable TKN concentration for a separate sludge system will be considerably slower than that of a combined sludge system.
- 3) Transfer function models together with time series models adequately describe both combined and separate sludge systems

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- operating under critical nitrifying conditions and are able to forecast the temporal variation in nitrification achieved.
- 4) The models developed for both separate and combined sludge systems indicate:
  - a) effluent filterable TKN concentration increases with filterable TKN loading but does not respond significantly to OC loading, and
  - b) effluent nitrate-N increases with filterable influent TKN concentration and decreases with influent filterable OC loading.
- 5) A greater effluent filterable TKN variation can be expected from nitrifying systems operated under variable flow and concentration inputs than for variable concentration inputs alone. The pseudo "steady-state" relationships can predict the daily mean effluent results based on flow-weighted sampling.



#### RECOMMENDATIONS FOR FUTURE WORK

- A detailed investigation at the pilot scale level aimed at developing design criteria for total nitrogen removal in combined sludge nitrification and denitrification systems.
- 2) Develop and demonstrate the applicability of dynamic models for nitrifying activated sludge systems over a wide range of process conditions (temperature, solids retention time, etc.). These models may take the form of simple empirical transfer function-noise models or may be composed of deterministic and stochastic functions.
- 3) Investigate the feasibility of employing simple transfer function-noise models to describe the dynamic behaviour of existing full-scale wastewater treatment plants and applying them as a tool for process control and design extension.

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# Deterministic Dynamic Model for SSC System

In modelling the SSC system, substrate and material balances are presented under the assumptions as presented in the section "Deterministic Dynamic Models."

### Substrate balances

a) Filterable degradable organic carbon:

$$F_1 = F_0 + F_{R1} - F_{W1}$$

$$F_{R1} - F_{W1} = F_1 - F_0$$
 .....(24)

equation (23) becomes:

$$V_1 = \frac{d S_1^c}{dt} = F_0 S_0^c - F_0 S_1^c - \frac{u^H X^H V_1}{v^H}$$
 .....(25)

where,  $u^H = K_G^H S_1^C$ , and

$$K_G^H$$
 = linear growth coefficient for heterotrophs  $(T^{-1})$ 

b) Ammonia nitrogen:

$$V_1 = \frac{d S_1^{NH} 4}{dt} = F_0 S_0^{NH} 4 + (F_{R1} - F_{W1}) S_1^{NH} 4 - F_1 S_1^{NH} 4$$

$$-\frac{u^{NS} x_1^{NS} v_1}{v^{NS}} - .14 \frac{u^H x_1^H v_1}{v^H} .....(26.)$$

using equation (24), equation (26) becomes:

$$V_{1} \frac{d S_{1}^{NH} 4}{dt} = F_{0} S_{0}^{NH} 4 - F_{0} S_{1}^{NH} 4 - \frac{u^{NS} X_{1}^{NS} V_{1}}{y^{NS}}$$

$$- .14 \frac{u^{H} X_{1}^{H} V_{1}}{y^{H}} \qquad (27)$$

where, the substrate utilization term uNS is expressed by:

$$u^{NS} = u^{*NS} \frac{S_1^{NH} 4}{(K_s^{NS} + S_1^{NH} 4)}$$

 $u^{*NS}$  = maximum growth rate coefficient  $(T^{-1})$ , and

 $K_s^{NS}$  = substrate concentration at one half maximum growth rate  $(M/L^3)$ .

In the above substrate balance, the rate of disappearance due to heterotrophic growth is estimated from considering the heterotrophs to be represented by the composition  ${\rm C_5H_7NO_2}$  (Hoover and Porges, 1952).

## c) Nitrite nitrogen:

$$V_{1} \frac{d S_{1}^{NO_{2}}}{dt} = F_{0} S_{0}^{NO_{2}} + (F_{R1} - F_{W1}) S_{1}^{NO_{2}} - F_{1} S_{1}^{NO_{2}}$$

$$+ \frac{u^{NS} X_{1}^{NS} V_{1}}{y^{NS}} - \frac{u^{NB} X_{1}^{NB} V_{1}}{y^{NB}} \qquad (28)$$

using equation (23), equation (27) becomes:

$$V_{1} \frac{d S_{1}^{NO} 2}{dt} = F_{0} S_{0}^{NO} 2 - F_{0} S_{0}^{NO} 2 - \frac{u^{NS} X_{1}^{NS} V_{1}}{v^{NS}}$$

$$- \frac{u^{NB} X_{1}^{NB} V_{1}}{v^{NB}} \qquad (29)$$

where the substrate utilization term  $\mathbf{u}^{\text{NB}}$  is expressed by:

$$u^{NB} = u^{*NB} \frac{S_1^{NO_2}}{(K_s^{NB} + S_1^{NO_2})}$$

 $u^{*NB}$  = maximum growth rate coefficient (T<sup>-1</sup>), and  $K_s^{NB}$  = substrate concentration at one half maximum growth rate (M/L<sup>3</sup>).

d) Nitrate nitrogen:

using equation (23), equation (29) becomes:

$$V_1 = \frac{d S_1^{NO_3}}{dt} = F_0 S_0^{NO_3} - F_0 S_1^{NO_3} + \frac{u^{NB} X_1^{NB} V_1}{v^{NB}}$$
 .....(31)

# Organism balances

Assuming the concentration of organisms in the feed stream is negligible  $(X_0 = 0)$ , mass balances for heterotrophs, <u>Nitrosomonas</u>, and <u>Nitrobacter</u> can be written as:

$$V_1 \frac{d X_1^H}{dt} = (F_{R1} - F_{W1}) X_{R1}^H - F_1 X_1^H + V_1 (u^H - K_D^H) X_1^H \dots (32)$$

$$V_1 = \frac{d X_1^{NS}}{dt} = (F_{R1} - F_{W1}) X_{R1}^{NS} - F_1 X_1^{NS} + V_1 (u^{NS} - K_D^{NS}) X_1^{NS} \dots (33)$$

$$V_1 = (F_{R1} - F_{W1})X_{R1}^{NB} - F_1 X_1^{NB} + V_1 (u^{NB} - K_D^{NB}) X_1^{NB} \dots (34)$$

where,  $K_D^H = \text{decay coefficient for heterotrophs } (T^{-1})$ ,

$$K_D^{NS}$$
 = decay coefficient for Nitrosomonas  $(T^{-1})$ , and

$$K_D^{NB}$$
 decay coefficient for Nitrobacter  $(T^{-1})$ .

Representing the settler performance by an efficiency  $E_1$ , operation is according to:

$$x_2^{NS} = (1.0 - E_1) x_1^{NS}$$
 .....(36)

$$x_2^{NB} = (1.0 - E_1) x_1^{NB}$$
 .....(37)

A solids balance around the settler yields:

$$F_1 X_1^H = F_2 X_2^H + F_{R1} X_{R1}^{NH}$$

and applying equation (35) leads to:

$$F_1 X_1^H = F_2 (1.0 - E_1) X_1^H + F_{R1} X_{R1}^H \dots (38)$$

The value of  $E_{1}$  can be determined according to:

$$E_1 = \frac{x_1 - x_2}{x_1} \qquad \qquad \dots \qquad (39)$$

Substituting equation (37) into the organism balance equation (31) and rearranging the result is:

$$V_1 = V_1 X_1^H (u^H - K_D^H) - F_{W1} X_{R1}^H - F_2 (1.0-E_1) X_1^H \dots (40)$$

The same expressions are applicable in terms of <u>Nitrosomonas</u> and <u>Nitrobacter</u>.

In summary, the important relationships describing the dynamic operation of the single-stage alternative appear in the information flow diagram (Figure 2).

#### Parameter Values for Dynamic Models

Before the equations which comprise the proposed dynamic models can be solved, values for the various parameters must be determined.

In the proposed models describing organic carbon degradation and resulting heterotrophic growth, values for the substrate utilization coefficient  $K_G^H$  and decay coefficient  $K_D^H$ , must be known or assumed. During steady-state simulation studies, Tan (1972), determined these values using a biological culture derived from an activated sludge plant treating the same raw sewage. Because the emphasis in this study is on nitrification, his values could be assumed sufficient for use in the organic carbon utilization and heterotrophic growth functions.

The  $u^*$  and  $K_s$  values in the Monod models for nitrification can be determined by various methods.

Determinations from pseudo "steady-state" studies, previously discussed, can be used to derive the parameter values. For example considering the single-stage carbon removal-nitrification alternative, at steady-state, equation 26 becomes:

where, 
$$u^{NS} = u^{*NS}$$
  $S_1^{NH*}$  ......(42)  
 $K_s^{NS} + S_1^{NH}$ 

substituting equation 41 into 40 and rearranging leads to:

$$\frac{T_1 X_1^{NS}}{S_0^{NH} 4 - S_1^{NH*} 4} = \frac{Y^{NS} K_S^{NS}}{u^{*NS}} \frac{1}{S_1^{NH*} 4} + \frac{Y^{NS}}{u^{*NS}}, \dots (43)$$

where, 
$$T_1 = \frac{V_1}{F_0}$$
 , and  $S_1^{NH}^*$  is the corrected effluent ammonia

concentration representing the concentration resulting if heterotrophic growth and subsequent nitrogen uptake did not occur. A graphical method can then be used to determine  $K_S^{NS}$  and  $u^{*NS}$  knowing  $Y^{NS}$ , from operating over a range of  $F_O$  values.

Batch studies can be used to determine the kinetic parameter values. For ammonia utilization by <u>Nitrosomonas</u>, the batch process equations are:

$$\frac{d \, S^{NH} 4}{dt} = \frac{u^{*NS} \, S^{NH} 4 \, X^{NS}}{Y^{NS} (K_S^{NS} + S^{NH} 4)}, \text{ and} \qquad .....(44)$$

$$\frac{d x^{NS}}{dt} = \frac{u * s^{NH} 4 x^{NS}}{K_s^{NS} + s^{NH} 4} - K_D^{NS} x^{NS}. \qquad (45)$$

The kinetic coefficients in these equations can be determined by a computer simulation of the unsteady-state equations. For a given set of initial parameter values, the simulation will produce curves of  $X^{NS}$  and  $S^{NS}$  versus time. Values of  $Y^{NS}$  and  $K^{NS}_D$  will be supplied as input and therefore are not part of the parameter set. Both the above batch and previously developed continuous time-dependent equations require initial values for the organism concentrations. Determination of these initial values can be afforded by developing a relationship, through batch experiments, between ammonia removal rate and percent Nitrosomonas present.

The above parameter determination procedures are applicable as well to the carbon and nitrite substrates involved and the corresponding heterotrophic and <u>Nitrobacter</u> organisms groups. A more direct procedure for determining an initial estimate of the heterotrophic population would be through the use of oxygen uptake measurements in which nitrification was inhibited (Toerber, 1972).

Applying kinetic coefficients determined from batch or continuous pseudo"steady-state"studies assumes that they are applicable to

transient conditions.

D

Another method of determining the parameters for the models developed, involves utilizing an optimization search routine along with the particular dynamic simulation method. The search routine determines the parameter values which give the best fit to the transient data.

Yield and decay coefficients for the nitrifying bacteria are quite small making experimental determination difficult. A review of the literature may be used to determine these values.

## Solids Retention Time (SRT) Calculation

The solids retention time for each reactor-clarifier system is defined as the solids in the aeration tank divided by the solids intentionally wasted or lost over the clarifier weir per day. During pseudo "steady-state" period the calculated SRT is based on a cumulative mean aeration tank MLSS concentration and total solids wasted or lost from the system. For the combined sludge systems (SSC, TSC, FSC), the final result is the system solids retention time (SSRT) for that pseudo "steady-state" period (Table B1). For the separate sludge system (TSS) the SRT for B1 and B2 were calculated in the above manner. The TSS system solids retention time (SSRT) was then calculated weighting the individual SRT's for B1 and B2 according to their aeration tank volumes (Table B1). The dates over which the SSRT was calculated during each pseudo "steady-state" period are indicated in the "Data Listing" section under "Mode of Operation" (Appendix B).

# Analytical Procedures

# Total kjeldahl nitrogen

Total kjeldahl nitrogen analyses (organic plus ammonia nitrogen) were performed according to Technicon Auto-analyser Industrial Method 146-71A. Essentially this procedure consists of digestion of organic matter at 380°C followed by measurement of the ammonia produced using the Berthelot reaction in which the formation of a blue indophenol complex occurs when ammonia reacts with sodium phenate followed by the addition of sodium hypochlorite. Glycine standards were used for calibration. For keeping unfiltered samples homogenized in the sample cups the system has two air aspirators. One aspirator provides complete mixing in the cup being sampled while the second aspirator mixes the next dup on the tray.

.BLE B1 SOLIDS RETENTION TIME CALCULATIONS - RUNS PSS - 21 AND - 22

TS	TSC SYSTEM												
Date	Aera A1	Aeration Tank MLSS g/l 11 A2 *A	k MLSS *A	†Cumul. Mean A	Mean Plant Solids g	Intent Waste Vol.	ional Wa Waste Conc. g/l	Intentional Wasting Waste Waste Solids Vol. Conc. Waste 1 g/1 g	Unintentional Wasting g	Total Waste g	Cumul. Total Waste 9	Cumul. SRT days	
(1974)	6.47	7.49	7.14	7.14	15593.8	45.5 47.8 95.5	10.26 9.92 11.03	466.8 474.2 1053.4	104.8	2099.2	2099.2	7.8	
18/6	5.94	5.795	5.84	6.49	14178.5	91.0	9.93	903.6	146.8	2489.7	4588.9	6.2	
19/6	5.91	5.5]	5.65	6.21	13562.6	95.5	9.19	877.6 661.8	188.7	1728.1	6317.0	6.4	
20/6		6.02 5.885	5.93	6.14	13409.8	113.8	10.13	1152.8	157.2	1310.0	7627.0	7.0	
Note:		weighted stands	MLSS ac for cum	*A is weighted MLSS according to Al +Cumul. stands for cumulative.	4	and A2 tank volumes.	olumes.						

TABLE B1 (Cont.d)

TSS SYSTEM: REACTOR B1

Aera	Aeration Tank MLSS g/l Bl Cumul. Mean Bl	MLSS Mean B1	Mean Plant Solids g	Intentional Waste Vol.	WASTING Waste So Conc. Wa	ING Solids Waste 9	Unintentional Wasting g	Total Waste g	Cumul. Total Waste 9	Cumul. SRT days
3.81	*	3.81	3017.5	159.3	8.69	1384.4	125.8	2869.7	2869.7	1.05
3,585	10	3.69	2928.4	136.5 204.8	6.94 9.93	947.3	325.0	3306.0	6175.7	0.95
3.69		3.69	2928.4	159.3 227.5	7.37	1174.0	146.8	2913.3	0.6806	0.97
3.35		3.61	2858.1	182.0 273.0	6.60 5.53	1201	52.4	2763.1	11852.1	*0.96

TABLE B1(Cont'd)

TSS SYSTEM: REACTOR B2

Cumul. SRT days 11.8 10.3 1529.6 361.9 8.006 1800.5 Cumul. Total Waste 275.9 361.9 538.9 623.8 Total Waste g Unintentional Wasting 125.8 109.8 188.7 83.9 174 (.96) + 306 (11.0) 480 WASTING +e Solids Waste 192.0 236.1 434.1 435.1 Waste Conc. 6.65 6.36 5.4] 6.37 ſ/g Intentional Note: \* TSS system solids retention time (SSRT) ≥ 35.5 35.5 68.3 68.3 5213.0 Mean Plant Solids 5693,3 4971.2 5303.5 Aeration Tank MLS. 9/1 02 Cumul. Mean B2 3.74 3,57 4.09 3.81 3.610 3.535 3.05 4.09 (1974)17/6 18/6 20/6 19/6 Date

#### Ammonia

Analyses of ammonia nitrogen were conducted using Technicon Auto-analyser Industrial Method 98-70W. This is essentially the same technique employed for Total kjeldahl nitrogen with the omission of the selenium dioxide/sulphuric acid/perchloric acid digestion step which ammonifies the organic nitrogen fraction. Ammonium chloride standards provided calibration.

#### Nitrite

Technicon Auto-analyser Industrial Method 100-70W was used for nitrite-nitrogen determinations. This technique involves a reaction between nitrite and sulphanilamide under acid conditions to form a diazo compound which in turn is coupled with N-1-naphthylethylenediamine to form a reddish purple azo dye. Colourimetric determination is then made on the sample.

## Nitrate plus nitrite

Nitrate plus nitrite-nitrogen analyses were performed using Technicon Auto-analyser Industrial Method 100-70W. In this method, the nitrate-nitrogen is reduced to nitrite in the copper-cadium reduction column. The sample is then analysed for nitrite nitrogen as described previously.

## Chemical oxygen demand (COD)

Early COD determinations were done according to the dichromate reflux method described in "Standard Methods" (1971). During the research period, a modified version of Technicon Autoanalyser Industrial Method No. 268-73W was adapted for COD analysis. A Technicon Solidprep 11 sampler was introduced in place of the normal sampler. This allowed analysis of samples containing suspended solids and provided high shear homogenization of samples with the dichromate and sulphuric acid reagents. Standard solutions were prepared using ammonium chloride. The standards were first analysed using the "Standard Methods" reflux technique and then analysed on the Technicon equipment. The standard peaks produced on the Technicon System were then calibrated against the "Standard Methods" results.

This complicated approach was necessary since the sample digestion time in the Auto-analyser was shorter than that in the standard reflux test. This resulted in a lower degree of reaction completion with the Auto-analyser when heterogeneous sewage samples were tested. With this procedure modification in effect, Auto-analyser COD results for sewage samples were generally only 5 to 7 percent lower than results obtained via the "Standard Methods" technique.

## Biochemical oxygen demand (BOD)

The 5 day, 20 degree C BOD determinations were performed according to the method described in "Standard Methods" pages 489 - 495 (1971).

## Filterable organic carbon (FOC)

Twenty micro-litre samples previously acidified and purged were injected into a Beckman Infrared Carbon Analyser. The resulting peaks were compared to a calibration curve prepared from standards using anhydrous potassium biphthalate.

#### Suspended solids

Gelman .45 micron glass fibre filters were dried, but not washed, for at least two hours in a 103 degree C oven. They were then cooled in a dessicator and weighed. Suspended solids determinations were made by filtering a minimum of 10 ml of solution through a filter. The filter was then re-dried at 103 degrees for two or more hours, dessicated for 15 minutes and re-weighed. The increase in weight was taken as a measure of the suspended solids.

#### Dissolved oxygen

An Electronic Instruments Ltd. Dissolved Oxygen Metre Model 15A was used for dissolved oxygen determinations. It was found necessary to calibrate the probe roughly once a week.

#### Temperature

The D.O. metre also included a temperature probe and this was used for measurement of the feed stream and the reactor

temperatures.

рΗ

pH was measured using an Orion Specific Ion Meter (Model 401) together with Fisher Combination electrodes (Cat. le-639-90).

#### Alkalinity

By using the Orion pH meter, 50 ml samples were titrated to a pH of 4.8 by addition of .02 N sulphuric acid. Results were expressed as mg/l as calcium carbonate.

# Data Listing - Mode of Operation, Reactor Operating Results, Analyses

The complete pilot plant data listing, composed of three sections, appears on the following pages. Contained under "Mode of Operation" is a chronological listing of information concerning pilot plant flow rates, operating reactor or mode, and raw sewage characteristics. This section also lists information on plant upsets, identifies acclimation and chemical addition periods, and notes the period over which the SSRT calculation was made for each plant. Contained under "Reactor Operating Results" is a chronological listing of the solids concentrations in the reactors, the waste concentrations and amounts, and other reactor characteristics such as pH, temperature, etc. Some clarifier effluent characteristics are also listed for the reactor system. Contained under "Analyses" is a chronological listing of analytically determined results for the various reactor streams.

The abbreviations and symbols used in the data listing are interpreted in Appendix G.

Mode of Operation

Note: Return Sludge and Feed Rates given as Igal/min.

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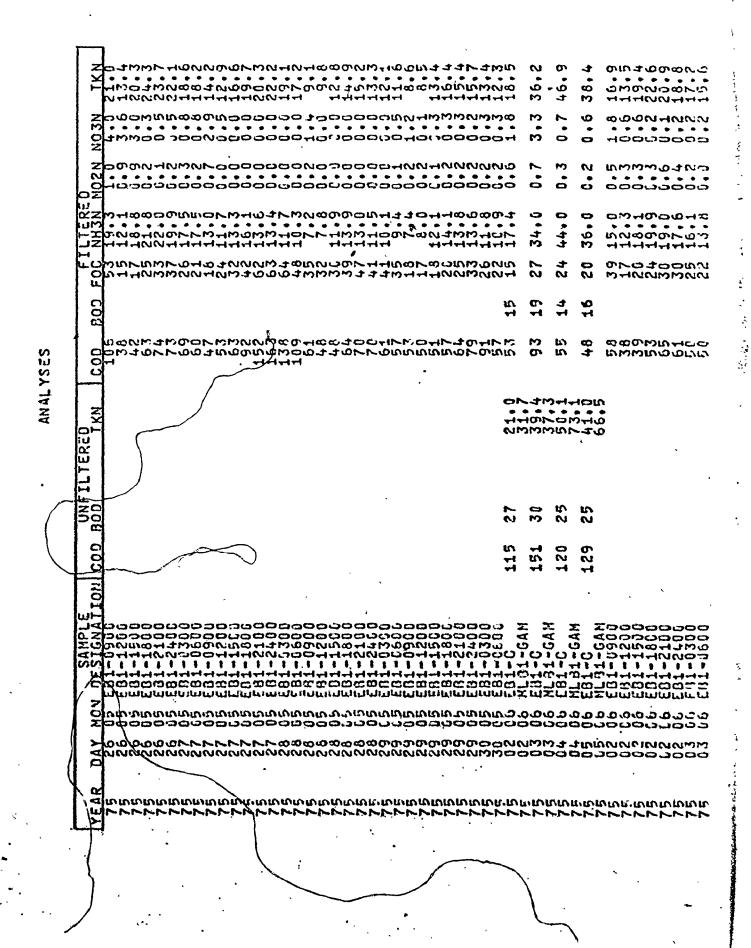
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#### APPENDIX C

#### Pseudo "Steady-State" Experimental Run Results

The results of experiments designed to compare the separate and combined sludge systems (Table 4) together with the results of other pseudo "steady-state" experiments undertaken to allow for system design are summarized in Table C1.

#### Genuine Versus Non-Genuine Repeated Results

A certain number of the pseudo "steady-state" experiments were repeated in order to allow estimation of the variance associated with the results. Certain repeated experiments were carried out during another pseudo "steady-state" period from that of the original experiment ("genuine" repeats), others during the same period (non-"genuine" repeats). In comparing the rate results (Table C2) there is no evidence to necessitate differentiating between "genuine" and non-"genuine" repeats.

#### Nitrification Rate and Ammonia Concentration Relationship

At  $15^{\circ}$ C and 4 day SRT the combined sludge system rate data indicates no dependence on ammonia concentration down to values approaching 0.5 mg/l NH<sub>4</sub><sup>+</sup>-N (Figure C1, Table C3).

# Effect of NO<sub>3</sub>-N on Analytical Determination of Filterable Organic Nitrogen

It has been stated (Parkin and McCarty, 1975) that the analytical determination of filterable organic nitrogen may be interfered with by the presence of  $NO_3^-$ N leading to an apparent lower value than actually present. The low concentrations of filterable organic nitrogen analytically determined in this research were independent of the concentration of  $NO_3^-$ N present (Figure C2).

#### Nitrification - HRT Relationship

The pseudo "steady-state" relationships developed between filterable TKN removal and temperature imply an independence on hydraulic residence time (HRT). The pseudo "stéady-state" models developed at a

PSEUDO "STEADY-SȚATE"EXPERIMENTAL RESULTS

TABLE CI

Run No.	Date	ta ta	System Conditions	System Sc	olids R	System Solids Retention	System	System Nitrification	ation	is'	ystem T	- 5
		င်္သ မ -	hrs hine hrs	SSC	.c TSS TSC	13C	SSC 9 F41	g Filterable T g MLVSS - day SC TSS	TKN TSC	SSC SSC	kemova: mg/l Filterab TKN SSC TSS T	emoval Filterable TKN TSS TSC
	(1973)											
PSS- 1	15/11	9	4	7	7		0.009	0.011		5.6	9.7	
PSS- 2	19/11	ഹ	4	7			900.0			3.9		
PSS- 3	20/11	ഹ	4	7	7		0.004	0.009		2.9	5.9	
PSS- 4	27/11	ហ	₩.			7			0.003			1.7
PSS- 5	11/62	ഹ	4			7			0.005			3.2
	(1974)											
PSS- 6	29/01	ശ	4	4	4		0.003	0.005		1.5	3.0	
PSS- 7	31/01	ស	❖	4	4		0.004	0.005		2.2	5.9	
PSS- 8	04/02	വ	₽*	<b>4</b> 7	**		0.005	0.008		3.0	ж. 8.	
PSS- 9	11/02	10	4		7	₹		0.030	0.026		15.1	16.3
PSS-10	19/02	7	4		7	4		0.013	0.011		5.7	5.1
PSS-11	21/02	10	4		<b>4</b> 3	4		0.021	0.020		12.1	11.2

TABLE C1 (Cont'd)

Run No.	Date	Syst Temp °C	System Conditions mp Detention Time C hrs	System S T SSC	Solids Ret Time days TSS T	System Solids Retention Time days SSC TSS TSC	System g Fil g ML SSC	System Nitrification Rate g Filterable TKN g MLVSS - day SSC TSS TSC	fication le TKN day TSC	Sy mg/1	System TKN Removal I Filterable TKN TSS TSC	KN able TSC
PSS-12	27/02	15	4		4	4		0.053	0.032		19.7	17.8
PSS-13	10/04	15	6.4		10	10		0.037	0.025		23.9	22.8
PSS-14	16/04	ഹ	6.4		10	10		0.022	0.014		16.5	15.4
PSS-15	18/04	rc	6.4		10	10		0.021	0.012	•	17.5	13.7
PSS-16	01/05	50	6.4	10	10		0.031	0.053		30.0	32.9	
PSS-17	21/02	ဖ	4		7	7		0.019	0.013		9.9	6.2
PSS-18	.04/06	7	ഹ	7	7		0.007	0.008		6.1	5. 8	
PSS-19	90/90	7	ഹ	7	7		0.0002	0.012		0.2	7.8	
PSS-20	13/06	10	വ	7	7		0.014	0.022		12.5	17.8	
PSS-21	18/06	10	ഹ		7	7		0.028	0.018		17.1	16.1
PSS-22	20/08	10	വ		7	7		0.037	0.021		17.9	18.4
PSS-23	28/06	5	ស		7	7		0.048	0.031		29.9	29.9

TABLE C1 (Cont!d)

Run	Date	Syst	System Conditions	System S	olids F	System Solids Retention	Syste	System Nitrification	cation	ν, ·	System TKN	Z Y
.00	ı	d S	herencion ime hrs	SSC	C TSS TSC	TSC	SSC 9	g Filterable TKN g MLVSS - day TSS TS	day TSC	mg/l	removal 1 Filterable TKN TSS TSC	able TSC
PSS-24	02/07	15	5	7	7		0.029	0.052		27.9	26.3	
PSS-25	05/07	15	Ŋ	7	7		0.029	0.068		28.4	27.4	
PSS-26	10/01	ø	വ		7	7		0.027	0.014		15.9	11.4
PSS-27	12/07	œ	വ		7	7		0.022	0.014		16.2	12.9
PSS-28	16/07	5	4	4	4		0.029	0.052		18.1	24.5	
PSS-29	18/07	15	4	4	4		0.031	0.049		23.1	25.6	
PSS-30	22/07	15	৺		4	7		0.0637	0.032		22.1	20.7
PSS-31	30/02	20	വ		7	7		0.086	0.044		38.1	40.1
PSS-32	80/90	50	Ŋ		7	7		090.0	0.036		35.6	35.1
PSS-33	80/80	20	ហ			7			0.030			28.9
PSS-34	13/08	50	ເດ	7	<b>,</b>		0.030	0.064		27.7	28.7	
PSS-35	15/08	50	ശ	7	7		0.040	0.082		32.8	32.4	
			,									

TABLE C1 (Cont'd)

No.	Date	Syst Temp °C	System Conditions Amp Detention Time C hrs	System S Ti SSC	System Solids Retention Time - days SSC TSS TSC	tention S	System g Fi	System Nitrification Rateg Filterable TKN g MLVSS - day SSC TSS TSC	ication e TKN day TSC	Sy mg/l SSC	System TKN Removal mg/l Filterable TKN SSC TSS TSC	CN 1ble TSC
PSS-36	20/08	12	2	7	7		0.026	0.051		22.2	20.8	
PSS-37	22/08	13	വ	7	7		0.033	0.058		29.6	30.4	
PSS-39	29/08	7	വ		7			0.016			11.2	
PSS-40*	60/90	25	5	7			0.035					
PSS-41*	10/09	52	Ŋ	7			0.034					
PSS-42	17/09	50	4			4			0.045			30.5
PSS-43	19/09	20	4			4			0.054			36.1
PSS-44	25/09	20	4		4	4		0.085	0.059		41.3	37.1
PSS-45	57/09	20	ಳ		43	\$		0.099	0.058		41.8	37.3
PSS-46	03/10	<b>∞</b>	വ		7			0.026			18.0	
PSS-47	08/10	∞	ហ		4			0.026			22.8	
PSS-48	10/10	∞	ហ		4			0.029			21.9	

24.3 21.8 73.1 33.4 180 mg/l Filterable System TKN Removal 63.9 33.4 26.8 56.4 76.6 32.5 22.1 15.1 82.6 12.4 32.1 SSC 0.020 0.038 0.057 0.062 0.032 0.057 g Filterable TKN g MLVSS - day SSC System Nitrification 0.069 0.129 0.164 0.150 0.213 0.068 0.039 0.050 0.038 0.077 0.024 0.060 0.048 0.022 0.034 0.071 System Solids Retention Time - days SSC TSS TSC 10 0 2 2 2 2 0 2 System Conditions Temp Detention Time °C hrs 6.4 6.4 6.4 6.4 20 25 25 25 25 23 2 2 0 0,0 5 27/11 (1975) 10/02 18/10 25/10 28/10 20/02 04/11 15/11 11/52 26/02 13/11 06/11 Date PSS-50\* PSS-51\* PSS-49 PSS-52 PSS-53 PSS-55 PSS-56 PSS-59 PSS-54 PSS-58 PSS-60 PSS-57

TABLE C1 (Cont'd)

TABLE C1 (Cont'd)

05/05       25       4       4       4       4       4       4       4       4       4       4       4       4       4       4       4       7.1         20/05       20       6.4       10       10       10       0.130       0.052	.0.	Ü		QET CCT	Ĭ,	200	1		27+0		4	- C - C - C - C - C - C - C - C - C - C	
05/05       25       4       10		)。		hrs	SSC	TSS	Tsc	F F	lterable	TKN	mg/l	Filtera	3ble
05/05     25     4     4     4     4     4     4     4     4     7.1       07/05     .25     4     4     4     4     0.261     0.135       20/06     20     6.4     10     10     0.130     0.052								SSC	TSS	TSC	SSC	TSS	TSC
07/05     .25     4     4     4     4       20/06     20     6.4     10     10     0.130     0.052	1		5	4	4	4		0.117			47.1	42.1	
20/06 20 6.4 10 10 0.130 0.052		•	LC C	4		4	4		0.261	0.135		42.6	49.8
	PSS-63 20/		0	6.4		10	10		0.130	0.052		48.9	47.8

TABLE C2 GENUINE VERSUS NON-GENUINE REPEATED EXPERIMENTS

Run No.	Date	Sys	tem Conditions	TSS System Solids	Nitrification Rate
,		Yemp. °C	Detention Time hrs	Retention Time days	g Filterable TKN g MLVSS - day
	(1974)				
PSS- 9	`11/2	10	5	7	0.030
PSS-18	4/6	7	5 5 5	7	0.008
PSS-19	6/6	7	5	7	0.012
PSS-20	13/6	10	5 /	7	0.022
PSS-21	18/6	10	5	7	0 <b>.0</b> 28
PSS-22	20/6	10	5	7	0.037
PSS-23	28/6	15	5	7	0.048
PSS-24	2/7	15	5	7	0.052
PSS-25	5/7	15	5	7	0.068
PSS-26	10/7	8	5	7	0.027
PSS-27	12/7	8	5	7	0.022
PSS-31	30/7	20	5	7	0.086
PSS-32	6/8	20	5	7	<b>0.0</b> 60
PSS-34	13/8	20	5	7	0.064
PSS-35	15/8	20	5	7	0.082
PS5-39	29/8	7	5	7	0.016
PSS-46	3/10	8	5	7	0.026

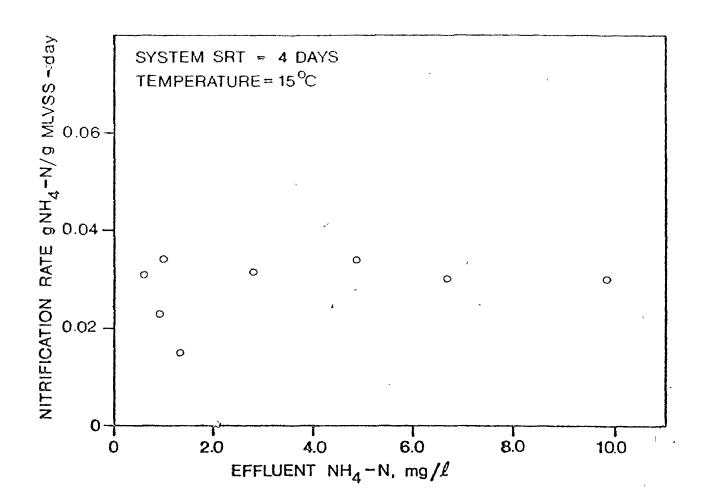


FIG. C1 INDEPENDENCE OF NITRIFICATION RATE AND AMMONIA CONCENTRATION

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TABLE C3 COMBINED SLUDGE SYSTEM RATE DEPENDENCY ON AMMONIA CONCENTRATION

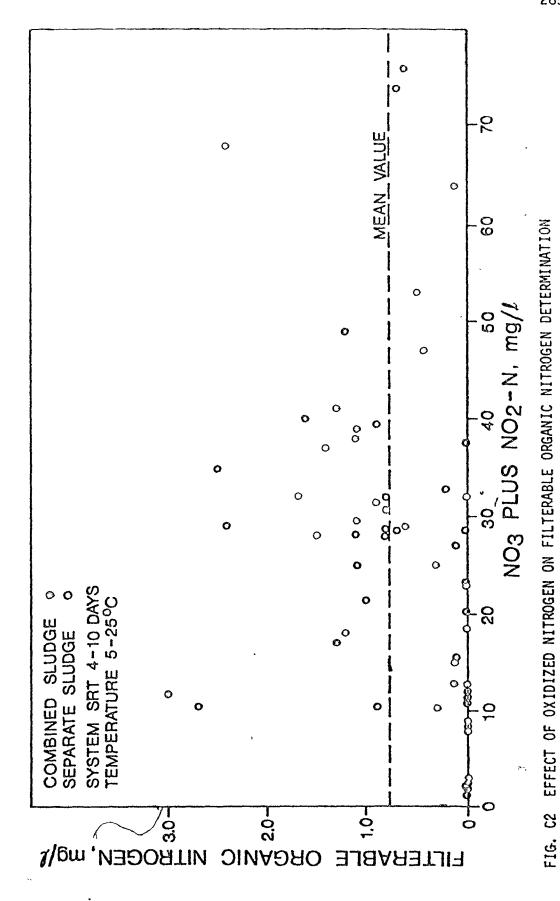
Run No.	Date	System Temp. °C	System Solids Retention Time days	Effluent NH <sub>4</sub> -N mg/l	Nitrification Rate g Filterable NHg-N g MLVSS - day
		Harris de la constante de la c			
	(1974)				
PSS-12	27/2	15	4 -	2.8	0.032
PSS-28	16/7	15	4	9.8	0.030
PSS-29	18/7	15	4	4.9	0.034
PSS-30	22/7	15	4	6.7	0.030
	(1975)		,**************************************	_	
* D-1	3/3	15	4	0.6	0.032
* D-2	11/3	15	4	1.0	0.034
* D-3	24/3	15	4	1.3	0.015
* D-4	7/4	15	. 4	0.9	0.023
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four day SSRT involved operating at an HRT of four hours. No difference in TKN removal was observed at an eight hour HRT under equal SRT and temperature conditions (Table C4.)

VERIFICATION OF FILTERABLE TKN REMOVAL - SRT-TEMPERATURE RELATIONSHIP TABLE C4

No.	Uate	System	System Conditions Temp HRT SRT °C hrs days	Condi HRT hrs	tfons SRT days	Filterable TKN Removal Observed **Model Prediction mg/l	KN Removal el Prediction mg/l
	(1975)						
9-0*	12/5	SSC	13	ω	4	19.9	16.3
4D-7	26/5	SSC	13	œ	<b>-</b> 3	15.8	16.3
*D-8	5/6	SSC	13	8	4	16.0	16.3
9-Q*	12/5	TSS	13	œ	4	21.5	20.2
<b>2-0</b> *	5/97	TSS	13	∞	4	18.9	20.2
*D-8	5/6	TSS	<u> </u>	∞	4	19.4	20.2
No te:	*Pseudo **Predict Combine Separat	Pseudo "steady~state" ba Predictions based on fil Combined sludge system: Separate sludge system:	state" bared on file system:	aselin Iterab K= 2 K= 2	le day c le TKN .52 x l	*Pseudo "steady-state" baseline day of dynamic run. **Predictions based on filterable TKN removal models Combined sludge system: K= 2.52 x 10 <sup>13</sup> e -15950/RT Separate sludge system: K= 2.98 x 10 <sup>10</sup> e -12000/RT	(HRT=4 hrs);

#### APPENDIX D

#### Mixing Characteristics of Reactors

In order to describe the mixing characteristics in the dispersed aeration tanks two flow models were used to interpret tracer response results.

#### Tanks in series model

The particular flow patterns which produce the effluent dye concentration curves in tracer studies can often be approximated by effluent concentrations predicted for a number of equal sized continuous stirred tanks in series (CSTR's).

The final effluent of a system of j equal sized CSTR's can be found from the following equation:

$$\frac{C}{C_0} = \frac{j^{j}(j\Theta)^{j-1}}{(j-1)!} e^{-j\Theta} \qquad .....(46)$$

where: C = effluent tracer concentrations

 $\Theta$  = dimensionless time

j = number of tanks

 ${\rm C_0}^{=}$  the quantity of tracer added divided by the volume of the entire system

This applies only to a pulse input of tracer. In this type of system, as j approaches large values (say 15), the flow regime approximates plug flow whereas, when j is equal to 1, the flow is completely mixed. If the time at which the peak dye concentration occurs is shown, the above equation can be solved for j by taking the derivative and equating the result to zero. Theta peak is determined by dividing the peak time by the residence time. The final form of the equation is:

#### Dispersion model

The dispersion model is developed in such a way that it assumes plug flow with the inclusion of a term which describes the degree of molecular dispersion or deviation from the ideal. The general equation for this model is:

$$-\frac{D^2C}{x^2} - \frac{UC}{x} - \frac{C}{x} = 0 \qquad .....(48)$$

Where: u = mean displacement velocity

C = concentration

 $\frac{C}{x}$  = concentration gradient

 $\frac{C}{+}$  = reaction term

D = turbulence expression

The solution of this equation for a tracer pulse input to a closed vessel given by Mujachi (1953) is quoted by Timpany (1966).

$$\frac{C}{C_0} = 2\tilde{\Sigma} \frac{U_n(U \sin U_n + U_n \cos n)}{(U^2 + 2U + U_n^2)} = EXP U - \frac{(U^2 + U_n^2)}{2U} = \dots (49)$$

where: 
$$U_n = COT^{-1} \left( \frac{U_n}{U} - \frac{U}{U_n} \right) / 2$$

$$U = \frac{uL}{2D}$$

L = tank length

The value  $\mathbf{U}_{\mathbf{n}}$  is best calculated by trial and error using an iterative approach. Also, the summation in equation forty-eight is taken to some reasonably large but finite value for practical purposes.

Instead of determining a value for D by the normal variance technique suggested by Levenspiel (1967), a correlation between peak time and D/uL developed by Timpany (1966) has been used. Proper use of the variance method for D/uL calculation generally requires concentration data to be entered to at least seven detention times.

This is rarely practical.

#### Mixing studies

The mixing characteristics of the single, two-stage, and five-stage combined sludge reactors (SSC, TSC and FSC) were established by the use of dye or tracer response methods. In addition the mixing regime in the first stage of the two-stage separate sludge system was determined.

All dye studies were conducted with a Turner Model 111 continuous flow fluorometer. The dye used in each run was prepared from 50% by weight stock Rodamine WT. Previous to the dye studies the mixed-liquor suspended solids were removed from the reactors and the testing conducted using tap water at approximately 8°C. Calibration curves were determined for the four fluorometer scales using dye solutions of known concentration at 8°C. To initiate each study a known slug of dye was added to the tank inlet through a tube immersed six inches below the liquid level. Halfway through the addition period (five sec ) the dye test was assumed to begin. The effluent dye concentration was measured at the outlet weir by pumping 280 ml/min of effluent through the fluorometer located within three feet of the sampling point. The fluorometer was connected to a Fischer Recordall Series 200 recorder.

The results and further details of each dye study are included in Tables D1, D2, D3 and D4 together with a listing of the computer program (Table D5) utilized in the analyses.

### TABLE DI TRACER RESPONSE ANALYSIS FOR SSC SLUDGE REACTOR (2/1/75)

TEST METHOD USING A PULSE INPUT OF RODAMINE WI DYE
REACTOR OPERATION AND TEST CONDITIONS
VOLUME OF REACTOR = 2184,33LITRES HYDRAULIC LOADING = 7.28LITRES/MIN'
THEOPETICAL DET = 3.5.34MIN  DYE INJECTION = 0.1843LITRES  CONC OF DYE ADDED = 1.238E+37PPP  DYE / TANK VOLUME = 2.2.51PPB
DIE 7 TANK VOLUME - 200 DIEFF
TEST RESULTS AND CALCULATED VALUES
DYE PEAK TIME = 4.30MIN PEAK/THEOR_DEI =C13
PEAK/MEAN DYE RES = 6.036MIN  MEAN DYE RESIDENCE = 110.034IN  PER DYE RECOVERY = 93.447#
FR. STAGNANT ZONE = J.633
CSTR S IN SERIES USING THEORETICAL RES. = 1.01  CSTR S IN SERIES USING MEAN DY RES. = 1.04
D/UL VALUE USING THEORETICAL RESIDENCE = 0.6516E+02  D/UL VALUE USING MEAN DYE RESIDENCE = 4.1697E+62

	٠

THETA	C/C7
0.160	5.989
2.20f 0.30f	5.817. 1.723
0.400	1.725 0.629
5 • 5 0 C	· 550
3.500	482
ŭ.760	(.419
2.80.	4.357
0.900	0.315
1.100	3.263
1.100	0.232
1.200	<u>°.201</u>
1.300	2.174
1.403	0.150
1.500	0.129
1.500	. <u>0.111</u>
1.700	ÿ • 199
1.300	1.186
1.900.	0.175
2.000	0.065
2.100	0.956
2.200	9.348
2.300	6.343
2.400	<u>0.93€</u>
2.500	3.331
2.501	6.127
2.700	. 124
2.800	<u> </u>
2.900	6.017
3.166	7.116
3.333	0.009
3.667 4.300	0.406
44 a.1 U U	1. 4 11 🔼 .

CALCULATE	$C \setminus CO$	 ERSUS	THETA	VAL	UES
		 SEPIE			

THEORETICAL DETENTION	ACTUAL DETENTION
THETA CŽCO	THETA C/CO

3.103		9 - 0 40	3.631
0.355		5.356	J.951
4.105	0.915		0.905
0.150	5.861	J. 150	0.861
7.200	1.819	0.2.0	1.819
0.250	J.779	r • 250	<b>3.779</b>
0.301	5.741	0.3.10	0.741
5.350	0.735	0.350	ٕ705
7.400	0.673	0.400	0.6.70
L.450	0.638	n • 450	9.638
2.500	1.617	0.500	0.607
0.550	0.577	0.550	0.577
0.660	7.549	2.6uD	0.549
0.650	0.522	0.650	0.522
<u> </u>	0.497	C. 7u6	i . 4 97
0.750	0.472	3.750	0.472
7.800	7.449	0.800	0.449
0.850	0.427	0.850	3.427
106.0	0.407	0.960	1.407
0.950	0.387	0.950	0.387
1.770	1.368	1.036	9.368
1.200	0.301	1.210	0.301
1.400	1.247	1.466	0.247
1.600	0. 202	1.600	0.202
1.860	1,155	1.810	0.165
2.380	0.135	2. 100	0.135
2.200	0.111	2.2:3	3.111
2.400	0.891	2 • + u 0	0.091
2.500	0.074	2.630	3.0.74
2.800	0.061	2.300	0.061
3.170	0.650	3.3.0	3.05)
3.209	0.041	3.210	0.041
3.400	1.033	3.400	0.033
3.699	0.027	3.600	9.027
3.80)		3.860	3.022
4.003	0.018	4.030	0.018
4.200		4.200	0.015
			****

CALCULATED C/CO VERSUS THETA VALUES FOR DISPERSION MODEL:

THEORETICAL D	ETEN TION	ACTUAL	DETENTION
2.162	0.909	2.140	0.922
9.209	0.823	9.2.0	ช • 8 3 3
398	0.744	2.310	0.753
t • 4,0 3	0.673	^ • 4 ± 0	7.681
<u> </u>	0.519	510	0.515
0.600	0.551	0.610	G • 5 56
<u>0.73r</u>	1.498	5.7.0	2.503
0.300	0.451	0.840	∂ • 4 55
206.3	7.478	2.916	0.411
1.306	3.359	1.000	0.372
1.100	3.334	1.1.0	0.335
1.200	0.302	1.200	0.304
1.300	0.273	1,300	6.274
1.400	0.247	1.400	0.248
1.500	0.223	1.5.0	0.224
1.600	3.292	1.630	0.203
	·		
1.780	0.193	1.700	9.183
1.800	0.165	1.8.0	0.165
1.900	0.150	1.950	0.151
2.000	0.135	2.038	3.135
2.199	0.122	2.110	0.122
2.200	0.111	2.200	0.111
2.300	0.110	2.356	3.103
2.400	0.031	2.430	0.093
2.500	1.632	2.5.6	0.082
2.500	0.674	2.600	
2.700	10.057	2.7.0	0.067
2.800	0.061	2.310	Q. 9% D
2.301	0.055	2.310	0.05

## TABLE D2 TRACER RESPONSE ANALYSIS FOR TSC SLUDGE REACTOR (6/1/75)

`
TEST METHOD USING A PULSE INPUT OF RODAMINE WY DYE
PEACTOR OPERATION AND TEST CONDITIONS
NOLUME OF REACTOR = 2184.QULITRES
HYDRAULIC LOADING = 7.28LITRES/MIN
THEORETICAL DET = 3.0.00MIN  OYF INJECTION = 0.1840LITRES
CONC OF DYE ADDED = 0.238E+07PPB  DYE / TANK VOLUME = 2.0.51PPB
TEST RESULTS AND GALCULATED VALUES
DYF PEAK TIME = 120.00MIN
PEAK/THEOR DET = 0.433
PEAK/MEAN DYE RES = U.636MIN  MEAN DYE RESIDENCE = 198.03MIN
PER DYE RECOVERY = 81.884#  FR. STAGNANT ZONE = 3.343
CSTR S IN SERIES USING THEORETICAL PES. = 1.67
CSTR S IN SERIES USING MEAN DYE RES. = 2.54
D/UL VALUE USING THEORETICAL RESIDENCE = 0.5875E+00 D/UL VALUE USING MEAN DYE RESIDENCE = 0.2179E+00

	THETA	CXC0	
		•	
	2.262		
	286.1	273	
	0.121	3 • 487	
	<u>`.18r</u>		
	2.240	3 • 7 € 8	_
	<u> </u>		
	€.350	J.772	
	<u> </u>	1.769	<del></del>
	0.480	<b>3.7</b> 59	
*	£.54°	1.746	
*	0.600	2.714	
	0.667	ū.571	
	9.720	3.620	
•			
	9.849	J.519	
	2.386	. 473	
·	3.360	3.43¢	
	1.126	0.393	
	1.980	0.363	
	1.145	7.336	
	1.200	0.292	,
	1.260	1.267	
	1.320	ù•24€	
	1.386	3.219	
<u> </u>	1.446	0.199	
	1.500	17.179	
	1.360	J.162	1
	1,620	0.144	
	1.580	0.125	
•	1.749	P : 112'	i
the state of the s	1.869	û.J98	
<b>{</b> 1	2.161	0.190	
	2.200		
2	2.400	J.344 	
<b>&gt;</b>	2.600		<del>i</del>
19	Z + O U U	9.912	

# CALCULATED C/CO VERSUS THETA VALUES FOR CSTR IN SERIES MODEL

	THECRETICAL	DETENTION	. ACTUAL DE	TENTION
	THETA	0.700	THETA	C/G0
	2,279	0.710	P.3.10	0.593
	0.150	J, 131	0.050	0.^29
		1.327	3.110	3 1 07
	2.15€~	0.444	(° 150 0 240	0 • 1 94 0 • 2 95
	<u> </u>	J.536 J.617	2.250	j.399
	.30:	3.659	0.3.0	1.494
	6.350	0.695	0.351	0.579
	( • 3 9 t	3.719	0.410	9.651
	0.+50	0.732	3.458	0.709
4			C 5 . C	0.753
	J.350	0.732	0.550	0.784
	2.500	3.723	0.6.0	1.803
	C.650	- 3.739	₹ • 656	0.811
		1.690	[.7.1)	0.813
	0.750	0.669	0.75v	3.833
	0.800	C . 545	0.830	0.784
	0.850	0.621	9.850	0.762
	<u> </u>	0.595	0.9:0	1.735
	0.350	0.568	0.950	3.705
-	1.300	C-541		0.6.72
	) 1.200	G • 435	1.200	0.531
	1.400	C. 341	1.4.6	1.397
	1.600	0.261 3.197	1.600 1.8:0	0.284 0.198
$\sim$ 7	2,303	0.147	2.008	3.134
	2.250 2.250	0.147 5.128	2.219	0.183
	2.400	0.079	2.436	0.058
	2.600	0.057	2.6.6	1.0.37
	2.800	0.041	2.8.0	0 <b>.</b> C 24
	3.111	1.037	3.017	7.715
	3.200	0.021	3.200	0.009
-	3.461	0.015	3.400	1.006
	3.600	0.311	3.500	0.004
	3.800	0.038	3.810	0.952
	4.700	0.005	4.000	3 • C 01
	4.268	0.004	4.230	0.001

CALCULATED C/CO VERSUS THETA VALUES FOR DISPERSION MODEL

THEORETICAL	DETENTION	ACTUAL	DETENTION
.155	0.124	C.1'C	0.01
. 3.203	0.612	0.200	1.197
	1.855	<u></u>	1.419
.405	J. 312	. 4.6	0.729
( 632	1.859	0.5:8 0.6:0	J.912
(.601 1.761	3.795 3.7±3	0.710	0.968
5 • 8 C C	0.623	0.8.0	3.865
3.000 3.000	1.55°	1.900	3.774
1.000	9.484	1.000	3.676
1.160	0.426	1.1.0	0.582
1.208	0.374	1.2.0	3.499
1.300	0.329	1.3.0	1.419
1.+00	0.289	1.400	J.353
<del></del>			
1.500		1.5.0	0.299
1.500 .	0.223	1.600	0.247
1.755	0.136	1.710	0.209
1.805	0.172	1.838	0.172 0.143
2.300	0.133	2.000	9.119
2.400	0.116	2.100	0.69
2.203	0.1)2	2.200	3.082
2.300	<u>n.sac</u>	2,310	1.059
2/+400	0.279	2.400	0.857
2.501	0.059	2.500	n - C 47
2.600	0.061	2.6.0	3 <b>.</b> r 39
2.700	1.653	2.740	0.133
		2.330	3.0 27
2.300	0.047	2.901	3.023

#### TABLE D3 TRACER RESPONSE ANALYSIS FOR FSC SLUDGE REACTOR (8/7/75)

REAC	OR OPERATION AND TEST CONDITIONS
	₩,
	VOLUME OF REACTOR = 2184 OULTRES
	HYDRAULIC LOADING = 7.28LITRES/MIN
	THEORETICAL DET = 300.00MIN
	DYE INJECTION = 0.0600LITRES
	CONC OF DYE ADDED = 0.2335+07PPB
	DYE / TANK VOLUME = 65.38PPB
TEST	RESULTS AND CALCULATED VALUES
V	
	DYF PEAK TIME = 180.00MIN
	DYE PEAK TIME = 180.00MIN PEAK/THEOR DET = 0.600
	DYE PEAK TIME = 180.00MIN PEAK/THEOR DET = 0.600
	PEAK/THEOR DET = 0.600
	PEAK/THEOR DET = 0.600  PEAK/HEAN DYE RES = 0.636MIN  MEAN DYE RESIDENCE = 262.53MIN  PER DYE RECOVERY = 114.304#
	PEAK/THEOR DET = 0.600  PEAK/HEAN DYE RES = 0.636MIN  MEAN DYE RESIDENCE = 262.53MIN
	PEAK/THEOR DET = 0.600  PEAK/HEAN DYE RES = 0.636MIN  MEAN DYE RESIDENCE = 262.53MIN  PER DYE RECOVERY = 114.304#
	PEAK/THEOR DET = 0.600  PEAK/HEAN DYE RES = 0.636MIN  MEAN DYE RESIDENCE = 262.53MIN  PER DYE RECOVERY = 114.304#
	PEAK/THEOR DET = 0.600  PEAK/HEAN DYE RES = 0.636MIN  MEAN DYE RESIDENCE = 262.53MIN  PER DYE RECOVERY = 114.304#
	PEAK/THEOR DET = 0.600  PEAK/HEAN DYE RES = 0.636MIN  MEAN DYE RESIDENCE = 262.53MIN  PER DYE RECOVERY = 114.304#
	PEAK/THEOR DET = 0.600  PEAK/MEAN DYE RES = 0.636MIN  MEAN DYE RESIDENCE = 262.53MIN  PER DYE RECOVERY = 114.304#  FR. STAGNANT ZONE = 0.125  CSTR S IN SERIES USING THEORETICAL RES. = 2.50
	PEAK/THEOR DET = 0.600  PEAK/TEAN DYE RES = 0.636MIN  MEAN DYE RESIDENCE = 262.53MIN  PER DYE RECOVERY = 114.304#  FR. STAGNANT ZONE = 0.125  CSTR S IN SERIES USING THEORETICAL RES. = 2.50  CSTR S IN SERIES USING MEAN DYE RES. = 3.18
	PEAK/THEOR DET = 0.600  PEAK/MEAN DYE RES = 0.636MIN  MEAN DYE RESIDENCE = 262.53MIN  PER DYE RECOVERY = 114.304#  FR. STAGNANT ZONE = 0.125  CSTR S IN SERIES USING THEORETICAL RES. = 2.50

THETA	C/CO	
		<del></del>
		•
0.075	0.314	<del></del>
0.150	D.115	
0.225	0.315	
	0.572	
0.375	0.772	
0.450	0.301	
0.525	0.973	
0.500	1.016	
€.675	1.002	
0.750	3.973	
0.825	0.916	
0.900	0.344	
0.975	0.801	
1.050	0.730	
1.125	0.658	
1.200	0.501	
1.275	0.529	
1.350	0.473	
1.425	0.430	
1.500	0.37.2	
1,575	0.329	
1.650	0.286	
1.725	0.257	;
1.800	C.214	
1.875	9.187	
1.950	0.165	
2.025	0.136	1
2.100	0.115	<del></del>
2.175	0.099	
2.250	<u> </u>	<del></del>
2.500	0.057	
2.750	- 0.029	

	CALCULATED C#CO VERSUS THETA VALUES FOR CSTR IN SERIES MODEL			
	•			
	THEORETICAL	DETENTION	ACTUAL D	TENTION
	THETA .	C/C)	THETA	C/C0
	0.000	0.000	0.000	0.000
	0.050	0.181	0.050	0.029
	0.100 0.15C	0.327	0.100 0.150	0.100
	0.206	0.536	0.200	0.296
<del></del>	0.250	0.607 0.659	0.250 0.300	0.399
	0.350	0.635	0.350	0.579
	0.400	0.719	0.400	0.651
	0.450	0.732	0.450	0.709
	0.500, 0.550	0.736	0.500 0.550	0.753 0.784
	0.500	0.723	0.600	0.803
	0.550	0.709	0.650	0.811
	0.700	0.690	0.700	0.810
	0.750	0.659	0.750	0.800
	0.800 0.850	0.646 0.621	0.800 0.850	0 • 7 84 0 • 7 62
	0.900	0.595	0.900	0.735
	0.950	0.568	0.950	0.705
	1.000	0.541	1.000	0.672
<del></del>	1.200	0.341	1.400	0.531 0.397
	1.600	0.251	1.600	0.33/
	1.800	0.197	1.800	Q • 1 93
	2.000	0.147	2.000	0.134
	2.200	0.108 0.079	2.200 2.400	0.089 0.058
	2.500	0.057	2.600	0.037
<del></del>	2.300	0.041	2.800	0.024
	3.000 '	0.030	3.000	0.015
<del></del>	3.200	0.021	3.200	0.009
	3.400 3.600	0.015 1.011	3.400 3.600	0.006 0.004
	3.800	0.008	3.800	0.002
<del></del>	4.000	0.005	4.000	0.001
	4.200	0.004	4.200	0.001

YALVIS	IS INTIA VA	ED C/CO VERSUN MODEL	FOR DISPERSI	
0 и	DETENTION	ACTUAL	DETENTION	THEORETICAL
	0.093	0.100	0.001	0.100
	0.021 0.207	9.230 9.306	0.1)7 0.432	0.200
	0.538	0.400	. 0.741	C + D C
	0.830	0.500	0.315	0.500
	0.995	0.600	0.465	0.500
	1.037	0.700	0.934	0.700
994	0.994	0.300	0.859	0.800
	0.903	0.300	0.766	0.300
	0.791	1.030	0.558	1.000
	0.676	1.136	9.575	1.100
	3.567	1.206	0.431	1.200
	0.469 0.385	1.300 1.400	0.416 0.350	1.300
	0.313	1.500	0.234	1.500
	0.254	1.000	0.247	1.600
	0.205	1.700	0.206	1.700
	_			
		4		•
And the second s				,
	0.164	1.800	0.172	1.800
	0 • 1 32	1.900	0.144	1.900
	0.105	2.000	0.120	2.000
	0 • 0 8 4 0 • 0 6 7	2.100 2.200	0.100 0.083	2.100
	0.054	2.338	0.069	2.300
	0.043	2.400	0.358	2.460
	0.034	. 2.500	.0.048	2.500
	0.027	2.500	0,040	2.500
	0.021	2.700	0.033	2.700
	0.017	2.810	0.028	2.300
	0.014	S•300	0.023	2.300
0.11	0.011	3.000	0.019	3.000
<del> </del>				
•	•	•	<i>*</i>	

# TABLE D4 TRACER RESPONSE ANALYSIS FOR FIRST STAGE OF. TSS SLUDGE REACTOR (7/1/75)

TEST METHOD USING A PULSE INPUT OF RODAPINE WI DYE
REACTOR OPERATION AND TEST CONDITIONS
VOLUME OF REACTOR = 796.33LITRES
HYDRAULIC LOADING = 7.23LITRES/MIN
THEORETICAL DET = 1.9.38 MIN
OYE INJECTION = 0.06JULITRES  CONC OF OYE ADDED = 0.238E+C7PPR
DYE / TANK VOLUME = 179.33PPB .
175. 3577 U.S.
TOOT OF OUR TO AND CALCULATED WALKES
TEST RESULTS AND CALCULATED VALUES
DYE PEAK TIME = 0.75MIN
PEAK/THEOR DET = U.CU/
PEAK/MEAN DYE RES = L.C11MIN
MEAN DYE RESIDENCE = 70.00 MIN
PER DYE RECOVERY = 112.166¢
FR. STAGNANT ZONE = 0.363
THE STANFALL COLC.
CSTR S IN SERIES USING THEORETICAL RES. = 1.61
CSTR S IN SERIES USING MEAN DYE RES. = 1.01
D/UL VALUE USING THEORETICAL RESIDENCE = (.1587E+33
D/UL VALUE USING MEAN DYE RESIDENCE = 1.8727E+U2

TABLE D4 (Cont'd)

THETA	0/00	
		€.
(.137	1.053	
v.274	2.305	
411	792	1
549	3.587	
v.686	. 589	
0.823	J.5[7	
0.960	J.429	
1 . 1 97	3.376	
1.234	2.317	ì
1.371	1.269	
1.508	J.236	
1.545	0.269	
1.783	5.175	
1.320	J • 1 4·8	
2.357	J.128	1
2.194	0.168	
2.331	0.093	
2.+68	3.089	
2.505	3.071	
2.743	J.J61	
2.380	2.053	
3.117	3.746	
3.154	0.949	
3.291	0.035	
3.428	0.036	
3.565	J.J25	
3.703	0.022	
3.340	v. U18	
3.977	1.017	
4.114	9.013	<del></del>
4.571	. u.u.s	
5.)28	<b>0.064</b>	
5.485	0.003	
2.402		

c<sub>a</sub>.a

## CALCULATED COOD VERSUS THETA VALUES: -

 FOR CSTP IN SERIES MODEL	
,	
 	<del>-</del>

	kandaga yan a shara sharayaran a sanayaran kadana ayanga da shara ayan da shara a sana da shara a shara da sha		
THETA	CACO	THETA	CVCC
5.252	J. J. E	5.356	3.733
6.350	J. 951	7. 150	1.951
J.160	0.9.5	0.106	J.905
0• <b>15</b> €	0.851	0.150	3.55.
1.201	0.319	? • 2 ⋅ 6	0.813
6.250	û.779	0.250	3.773
L.3(L	3.7-1	9.366	3.741
0.350	<b>0.7</b> u5	ۥ350	J.765
J.403	J.67ú	C • 4J0	J.673
₩• <b>→</b> 50	0.638	0.450	J • 6 38
6.509	3.607	0.500	3.607
i • 55€	u•577	9.550	u • 5 77
0.600	J.549	0.6.0	J.543
J.653	0.522	0.650	J.522
6.785	0.497	C.7JU"	J.497
€.750	0.472	0.750	0.472
3.300	0.449	0.800	3.443
0.859	0.427	<b>₢</b> • 850	3.427
0.900	0.4.7	9.9.9	5.437
0.959	0.387	0.95ุง	0.387
1.J0C	0.368	1.000	3.363
1.200	0.301	1.230	0.301
1.400	ù.2+7	1.400	0.247
1.600	0.232	1.636	J.202
1.300	0.165	1.8.1	0.165
2.300	9.135	5 • 0 - 0	J • 1 35
2.200	U.111	2,211	Ú.11i
2.40	9.091	2.430	0.691
2.600	Ú• 074	2.6.0	0.074
2.800	0.61	2 • 8 • 3	3.€61
· 3.JUU	<b>Ú.</b> 050	3.000	0.051
3 • 26 b	0.641	3.200	J.041
3.400	0.033	3.400	0.1.33
3.500	0 • ú 27	3.600	3.027
.3.300	0.022	3.8.0	U.U22
4.100	0.018	4.030	0.018
4.200	0.015	/ 4.230	0.015

	4. 4.	ATFO C/CO	 THETA	VALUES
FOR	गाडन्ट रड	रावा पटार		

THEORETICAL	NCI THETEG	ACTUAL DE	LINTION
		٧,	
5.166	J. 317	1.130	9.978
1.200	0.820	0.200	J. 822
3.366	1.742	<del>3.3</del> 30	3.743
0.400	0.671	`.400	0.672
	1.5.7	7.520	3.608
0.600	0.550	7.6.0	ú.55)
731	J.437	730	. 3.498
.86€	3.456	€ • 800	).45,
1.06.5	3.407	J. 900	J • 4U/
1.765	3.358	1.500	J.369
1.109	3.333	1.130	ü • 3 33
1.200	0.331	1.2.8	0.302
1.300	U•273	1.300	3.273
1.498	0.247	1.400	0.247
1.500	0.223	1.500	0.223
1.600	J. 2J2	1.610	0.202
1.700	0.183	1.7.0	9.183
1.800	0.165	1.330	0.165
1.903	9.156	1.910	J.15J
2.000	0.135	2.368	0.135
2.100	6.122	2.100	0.122
2.200	0.111	2.230	0.111
2.330	0.109	2.3.0	0.100
2 • 4 8 3	. 0.091	2 • 4 • 0	3.091
2.500	<b>J.</b> U82	2.5.0	0.082
2.600.	0.074	2.630	9.674
2.710	J. 957	2.700	3 • 6 67
2.800	0.06i	<b>2.8</b> 50	0 • C 61
2.905	0.055	2.9.0	3.055
3.300	J.050	3.468	3.053

## TABLE D5 TRACER RESPONSE ANALYSIS COMPUTER PROGRAM

 THE TWO CHIEF REFERENCES USED FOR THIS PROGRAMME ARE
 1. LEVENSPIEL , CHEMICAL REACTION ENGINEERING , CHAPTER 9
THE THE THE THE THE THE THE THE THE THE
 2. TIMPANY, VARIATION IN AXIAL MIXING IN AN AFRATION
 TANK. MASTERS THESIS, DEPT. OF CHEM ENG., MCMASTER UNIVERSITY. 1966.
 UNI 15 1 30 3 4 1 30
 THE DAME VALUE FOR THE DISPERSION MODEL IS SOLVED BY USING THE
CORRELATIONS OF PEAK TIME VERSUS D/UL DEVELOPED BY TIMPANY. (PP 31 -
THE CSTR IN SERIES MODEL IS SOLVED BY TAKING THE DERIVATIVE OF
 EQUATION 9-35 IN LEVENSPIEL - EQUATING THE RESULT TO ZERO AND
SOLVING FOR THE NUMBER OF EQUAL TANKS IN SERIES, J , IN TERMS
 OF THETA. THETA IS FOUND BY DIVIDING THE PEAK DYE TIME BY THE THEORETICAL RESIDENCE TIME.
THE C/CO VALUES FOR THE DISPERSION MODEL ARE SOLVED BY ITERATION USING EQUATION & IN CHAPTER 2 OF TIMPANY.
 THE CACO VALUES FOR THE CSTR IN SERIES MODEL ARE SOLVED USING
EQUATION 9-35 IN LEVENSPIEL FOR VARIOUS VALUES OF THETA.
 DIMENSION ( (503), CUL (500), TANKS (5), AW (5), DUL P (5), THETA (2,100)
DIMENSION CCO(2,100),U(500),AMU(5,500),COCO(5,500),CCI(100)
 DIMENSION TTB(100), TBAR(2), RATIO(500), BETA(500), ETA(500) DIMENSION BLUE(100)
 DIMENSION DECITORS
 VOLT=TANK VOLUME IN LITRES VELR=FLOW IN LITRES/MIN
TPEAK=PEAK TIME IN MINUTES DYIN=AHOUNT OF DYE IN LITRES
 DYCON=CONC OF DYE IN PPR DT=MINUTES BETWEEN DATA PIS
N=NUMBER OF DATA PTS
 C(I)=COUC OF DYF IN FFFLUENT PPR
DEAD A HOLT HELD TOTAL DATA DATA DATA
 READ 1, VOLT, VELR, TPEAK, DYIN, DYCON, DT PRINT 1, VOLT, VFLR, TPEAK, DYIN, DYCON, DT
READ 2.N
 PRINT 2,N
 READ 3. (C(I), I=1.N)
PRINT 3.(C(I).I=1.N)
 ,
 PERCENT DYE RECOVERY
AMT= C(1)*DT*VFLR*10.**(-6) CUL(1) = AMT
 DO 100 I=2, N
 AMT=.5*(C(I)+C(I-1))*DI*VFLR*10.**(-6)
LUL = I-1 CUL(I) = CUL(LUL) FAMT
 0 CONTINUE

```
PER = GUL(N)/)YE*130.
     CALCULATION OF MEAN RESIDENCE TIME OF THE TOTAL DYE RETRIEVED
C
    CALCULATION OF PERCENT STAGNA IT ZONE
_C
С
TBAR(1) = VOLI/VELR
     ANT = 1.
    I = 1
     ZONE = CUL(N)/2.
 201 IF (CUL(I).GT. ZONE) GO TO 200
     ANT = ANT + 1.
     I = I + 1
     GO TO 201
 2CO TBAR(2) = ANT-OT
     DEAD = (TBAR(1)-TBAR(2))/TBAR(1)
_
     CALCULATION NUMBER OF TANKS IN SERIES
C
<u>_</u>
     TP1 = TPEAK/T3AR(1)
     IP2 = IPFAK/IBAR(2)
     TANKS(1) = 1./(TP1*(1./TP1-1.))
     TANKS(2) = 1./(TP2+(1./TP2-1.))
C
    TRUNCATE TO NEAREST WHOLE NUMBER OF TANKS
_C_
C
     AW(1) = TANKS(1)
     AA = 1.5
  203 IF (AN(1) . 1 T. AA) GO TO 202
     AA = AA + 1.
     GO TO 203
  202 \text{ AW}(1) = AA - .5
     \Delta W(2) = IANKS(2)
     AA = 1.5
  205 IF (AH(2) ALT AA) GO TO 204
     AA = AA + 1.
     60 IO 205
  2C4 AW(2) = AA - .5
T
C
     CALCULATION OF TIMPANY & PEAK TIME D/UL VAULES
ᡗ
     IF ((TP1.GT.0.03).AND.(TP1.LT.0.3)) GO TO 206
     IF ((TP1.GT.Q.3).AND.(TP1.LT.Q.81) GO TO 207
     GO TO 208
296 DULP(1) = .2*(TP1**(-1.34))
     GO TO 209
  207 DUEP(1) = 4.027*(10.**(-2.09*TP1))
     GO TO 209
 208 PRINT 330
  IF (TP1.LE.0.03) GO TO 206
     GO TO 207
  209 CONTINUE
```

```
IF ((IP2.GI. 0. ú3). AND. (IP2.LE. 1.3)) GO TO 2\d
      IF ((TF2.GT.J.3).AND. (TP2.LE.U.8)) GO TO 211)
     GO TO 213
  210 \text{ DULP}(2) = .2*(T-2*+(-1.34))
  GO TO 214
  211 DULP(2) = 4.027*(10.**(-2.09*TP2))
GO TO 214
  213 PRINT 300
IF (IP2.1=.0.03) GO TO 210
      GO TO 211
  21 CONTINUE
C
     CALCULATION OF C/CO VS THETA VALUES FOR CSTR MODELS
ſ
C
      DERIVATIVE AT PEAK DYE CONC METHOD USED
C...
      DO 181 I=1.2
     X = \Delta W(T) \times \Delta W(T)
      XX=1.
      RR=1.
  216 IF (AW(I).EQ.B3) GO TO 215
      XX = XX^* (AH(I) - BB)
      88=38+1.
      GO IO 216
  215 \text{ FACT} = XX
      THETA(I.1) = 0.
      00 \ 102 \ J=2,21
     THETA(I,J) = I+ETA(I,J-1) + .05
  102 CCO(I,J)= X/XX*THETA(I,J)**(Aw(I)-1,)*EXP(-Aw(I)*THETA(I,J))
      00 103 J= 22.37
      THETA(I,J) = THETA(I,J-1) + .2
  103 CCO(I.1) = X/XX*THFTA(I.1)++(AW(I)-1.)*EXP(-AW(I)*THETA(I.J))
  101 CONTINUE
ع
      CLACULATION OF ACTUAL C/CO VALUES FROM EXPERIMENTAL DATA
C
\mathbf{r}
      CNOT = DYIN*DYCON/VOLT
      MM = 0
      DRAG = 0.
      NN = 0
      00\ 104\ I = 1,30
      NN = NN + 3
      MM = MM + 1
      RATIO(MM) = C(NN)/CNOT
      DRAG = DRAG + 3*DT
      RETAINM = DRAG/TBAR(1)
  104 ETA(MM) = DRAG/TBAR(2)
     IM = (N - 90)/10 + 29
      00\ 105\ I = 31, IM
      MM = MM + 1
      DRAG = DRAG + 10.*DT
    NN = NN + 10
      RATIO(MM) = C(NN)/CNOT
```

```
BETA(MM) = ORAG/IBAR(1)
      ETA(MM) = DRAG/CNOT
105 CONTINUT
      CALCULATION OF CACO VALUES VS LHETA FOR DAUL METHOD
C
      M = 1
____4J_<u>I=1</u>___
      AMU (11, I) = 1.4
      U(M) = .5/0ULP(M)
   45 \text{ AMU(M,I)} = \text{AMU(1,I)} - .001
      FR= COS(AMU(N,I))/SIN(AMU(M,I))
      FR= FR - AMU(4, I) *DULP(4) + .25/(AMU(M, I) * DULP(M))
      IE (FR) 45.45.51
   50 \text{ AMU}(M, I) = AMU(M, I) + .00001
      FR = COS(AMU(Y,T))/SIN(AMU(Y,T))
      FR = FR - AMU(4,I)*DULP(M) + .25/(AMU(M,I)*DULP(M))
      IF (FR) 55.50.51
   55 \text{ AMU}(M,I) = \text{AMU}(M,I) - .0000001
      FR = COS(AMU(4.T))/SIN(AMU(4.T))
      FR = FR - AMU(4,I) + OULP(M) + .25/(AMU(M,I) + OULP(M))
      IF(FR) 55.55.60
   60 I = I + 1
      \Delta MU(M,T) = \Delta MU(M,T-1) + 3.1417
      IF(I.LE.50) GO TO 45
      M = M + 1
      IF(M.LE.2) GO TO 40
      00 80 4=1.2
  999 ZETA = 0.0
      00 70 K=1.30
      ZETA = ZETA + .1
      COCO(M_{\bullet}K) = 0.0
      DO 65 I=1,50
      A=2.0*AMU(M.I)*(U(M)*SIN(AMU(M.I)) + AMU(M.I)*COS(AMU(M.I))
      B=EXP(U(M)-((U(M)**2 + AMU(M,I)**2)/{2.0*U(M)))*ZETA)
      D=U(M)**2 + 2.0*U(M) + AMU(M.I)**2
  998 CE(I) = A*B/D
      COCO(M \cdot K) = COCO(M \cdot K) + CE(I)
   65 CONTINUE
   F7 CONTINUE
   70 CONTINUE
   80 CONTINUE
C
      PRINT INSTRUCTIONS AND DATA PRESENTATION FORMAT
T.
C
      PRINT 700
  700 FORMAT(42X, 24HTRACER RESPONSE ANALYSIS///)
      PRINT 701
  7C1 FORMAT (40 X, 29 HROTATING BIOLOGICAL CONTACTOR)
      PRINT 702
  702 FORMAT (42X, 26 HHYDRAULIC CHARACTERIZATION///)
```

```
PPINI 733
  703 FORMAT(30X, *TEST METHOD USING A PULSE INPUT OF ROBAMINE WT DYE#,/)
  704 FORMAT(23X, 37HREACTOR OPERATION AND TEST CONDITIONS///)
    PRINT 765, VOLT
  705 FORMAT (31X, 20HV) LUNE OF REACTOR = ,F7.2,6HLITRES)
PRINT 795.VELR
  705 FOR MAT (C1X, 23 HHYDRAULIC LOADING = ,F7.2, 19HLITRES/MIN)
PRINT 707.TJAR(I)
  707 FORMAT (31x, 2LHTHE ORETICAL DET = ,F7.2,3HMIN)
    PRINT 718-DYI'I
  703 FORMAT (31X, 20HOYE INJECTION
                                     = ,F7.4,6HLITRES)
  PRINT 719.DYCON
  709 FORMAT(31X, 20HCONC OF DYE ADDED = ,E10.3,3HPPB)
   PRINT 710.C'OT
  710 FORMAT (31X, 20 HDYE / TANK VOLUME = ,F7.2,3HPPB,///)
      PRINT 711
  711 FORMAT (23X, 34HTEST RESULTS AND CALCULATED VALUES,///)
      PRINT 712 IPFAK
  712 FORMAT (31X, 20 HOYE PEAK TIME
                                     = ,F7.2,3HMIN)
      PRINT 713-TP1
  713 FORMAT(31X, 20HPEAK/THEOR DET
                                     = .F7.3/)
     PRINT 714.TP2
  714 FORMAT (31X, 20HPEAK/MEAN DYE RES = ,F7.3, 3HMIN)
      PRINT 715, TBAR(2)
  715 FORMAT (31X, *MEAN DYE RESIDENCE = #, F7.2, *MIN*)
     PRINT 716.PFP
  716 FORMAT(31X, 20HPER DYE RECOVERY = ,F7.3,1Hr)
      PRINT 717, DEAD
  717 FORMAT(31X, 20HFR. STAGNANT ZONE = ,F7.3,/)
     PPINT 718 TANKS(1)
  718 FORMAT(31X,42HCSTR S IN SERIES USING THEOPETICAL RES. = .F7.2)
      PRINT 719 TANKS(2)
 713 FORMAT (31X, 42HCSTR S IN SERIES USING MEAN DYE RES. = ,F7.2)
     PPINT 720 DUI P(1)
  720 FORMAT (31X, 424D/UL VALUE USING THEORETICAL RESIDENCE = ,E11.4)
      PRINT 721 DULP(2)
 721 FORMAT (31X, 42HO/UL VALUE USING MEAN DYE RESIDENCE
                                                             = ,E11.4)
      PRINT 722
 722 FORMAT (31X, 38HEXPERIMENTAL RESULTS C/CO VERSUS THETA, ////)
 723 FORMAT (14X, 5HTHETA, 15X, 4HC/CO, ///)
     PRINT 724, (BETA(I), PATIG(I), T=1, MM)
  724 FORMAT (15X, F5.3, 15X, F5, 3)
     PRINT 725
  725 FORMAT (31X, 35HCALCULATED C/CO VERSUS THETA VALUES)
      PRINI 726
  726 FORMAT (37X, 24HFOR CSTR IN SERIES MODEL, ////)
     PRINT 727
  727 FORMAT (12X, #THEORETICAL DETENSION
                                                    ACTUAL DETENSION#,//)
      PRINT 728
  728 FORMAT (15X, 5HTHETA, 6X, 4HC/CO, 15X, 5HTHETA, 6X, 4HC/CO, ///)
```

	PRINT 729, (THETA (1, J), CCO (1, J), THETA (2, J), CCO (2, J), J=1,37)
723	FORMAT (15X, F5.3, 6X, F5.3, 15X, F5.3, 6X, F5.3)  PRINT 725
	PPINT 730
733	FORMAT (26X, 20HEOR DISPERSION MODEL,///)
	PRINT 727
	BLUE (1) = .1
	00 731 K=2,30
	BLUE (K) = BLUE (K-1) + 1
731	CONTINUL
	PPINT 729, (3LUE(K), COCO(1, K), BLUE(K), COCO(2, K), K=1, 3C)
1	FORMAT (4F10.4,E10.2,F10.4)
2	FORMAI (I10)
3	FORMAT (5F10:2)
300	FORMATILIEX, 44 HPCAK TIME OUTSID: LIMIT FOR DAUL CALCULATION
	210 <sub>0</sub>
	ENO.

## Effect of SRT and Temperature Variables on Nitrification

The complete factorial design at five levels of temperature and three levels of SRT (Table 10) allowed the effects of these factors on ~ the output responses (filterable TKN removal and unit rate of filterable TKN removal) to be determined through an ANOVA (Tables El and E2). The factorial design, replicated once, allowed determination of the main effects of each variable and the two-factor interaction (Tables 11 and 12). SRT and temperature being quantitative factors, made it desirable to examine the relationship between the observed responses and the levels of each factor. The relationship between the response and the level of the factor can normally be approximated over a finite range of the factor by means of a polynomial. When the levels of the factor are at equal intervals the components of the polynomial linear, quadratic, etc., are all orthogonal to one another facilitating their determination. Calculating the components of the main effect of each factor (SRT and temperature), effectively gives the average shape of the curve relating the response to the level of the factor. It is possible for the shape of the curve describing the relationship between the response and one factor to change from one level of the other factor to another. This will be revealed by a detailed analysis of the two-factor interaction. For example, from Table E2 the interaction analysis for the combined sludge systems, SSC and TSC, reveals significant interactions of linear A (SRT) with linear B (Temperature) and quadratic B indicating the linear effect of A differs for different levels of B.

The methods and calculations involved in preparing the detailed ANOVA's are outlined in Design and Analysis of Industrial Experiments (1956).

## Pseudo "Steady-State" Reparameterised Arrhenius Models

The analysis of variance results for the reparameterised Arrhenius models, describing the unit rate of filterable TKN removal and the filterable TKN removal as functions of temperature (Figures 13, 14, 15, 17, 18, and 19), were established from the use of a non-linear least squares routine. An example of the use of the computer program including the output results appears in Table E6. The sum of squares after regression

TABLE E1 DETAILED EFFECTS OF SYSTEM SRT AND TEMPERATURE ON FILTERABLE TKN REMOVAL IN SEPARATE AND COMBINED SLUDGE SYSTEMS

٠,٥

Source of Variation	Sum of Squares	Degree Freed		Mean Square (MS)	Variance Ratio (MS/MSPE)	F α=.9
Main Effects						
SRT(A):			•			
TSS	935.64	2		467.82	14.49	3.58
Components - Linear	907.21	j		907.21	28.10	4.54
- Quadratic	28.43	ı		28.43	0.88	4.54
SSC or TSC	886.20	2	<b>₹</b>	443.10	15.10	3.68
Components - Linear	869.88	]	,	869.88	29.65	4.54
- Quadratic	16.32	ì		16.32	0.56	4.54
Temperature (B):		,				
TSS	7339.02	4		1834.75	56.84	3.06
Components - Linear	7078.55	1		7078.55	219.29	4.54
- Quadratic	142.88	1		142.88	5.43	4.54
- Cubic	83.08	1		83.08	2.57	4.54
- Quartic	34.51	1		.34.51	1.07	4.54
SSC or TSC	10091.20	4		2552.80	85.96	3.06
Components - Linear	9370.00	1		9370.00	319.36	4.54
- Quadratic - Cubic	527.00 153.28	1		527.20 153.28	17.96 5.22	4.54 4.54
- Quartic	46.92	i		40.92	1.40	4.54
·	,			,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	***	
SRT-Temperature Interactions:	264 07	•		45.50	3 43	0.64
TSS	364.97 44.95	8 1		45.62 44.95	1.41 1.39	2.64 4.54
Components - Linear A x Linear B - Linear A x Quadratic B	208.29	1		208.29		4.54
- Quadratic A x Linear B	1.05	i		1.05	0.03	4.54
- Quadratic A x Quadratic B		i		18.40	0.57	4.54
- *Remainder	92.28	4		23.04	0.71	3.06
SSC or TSC	442.01	8		55.25	1.88	2.64
Components - Linear A x Linear B	110.89	1		110.89	3.77	4.54
- Linear A x Quadratic B	189.44	1		189.44	6.46	4.54
- Quadratic A x Linear B	6.26	]		6.26	0.21	4.54
<ul><li>Quadratic A x Quadratic B</li><li>*Remainder</li></ul>	63.15 72.27	4		63.15 72.27	2.15 0.62	4.54 3.06
- "Relia Muer"	16.21	*		12.21	0.02	3.00
Pure Error (PE):						
TSS	484.23	15		32.28	-	-
SSC or TSC	440.15	15		29.34	••	-
Note: *Remainder = Cubic B x Li Cubic B x Qu Quartic B x Quartic B x	adratic A Linear A	•				

TABLE E2 DETAILED EFFECTS OF SYSTEM SRT AND TEMPERATURE ON UNIT RATE OF FILTERABLE TKN REMOVAL IN SEPARATE AND COMBINED SLUDGE SYSTEMS

Source of Variation	*Sum of Squares	Degrees of Freedom	*Mean Square (MS)	Variance Ratio (MS/MSPE)	F α=.95
Main Effects					
SRT(A): TSS Components - Linear	2.64 0.24	2	1.32	2.75 0.50	3.68 4.54
- Quadratic SSC or TSC	2.40 1.56	1 2	2.40 0.78	5.00 13.00	4.54 3.68
Components - Linear - Quadratic	0.74 0.82	1	0.74 0.82	12.30 13.70	4.54 4.54
Temperature (B): TSS Components - Linear - Quadratic - Cubic - Quartic	125.98 103.65 17.17 4.54 0.62	4 1 1 1 1 1	31.50 103.65 17.17 4.54 0.62	65.61 215.90 - 35.77 9.46 1.29	3.06 4.54 4.54 4.54 4.54
SSC or TSC Components - Linear - Quadratic - Cubic - Quartic	18.84 16.80 1.55 0.46 0.03	4 1 1 1 1 1	4.71 16.80 1.55 0.46 0.03	78.50 280.00 25.83, 7.67 0.50	3.06 4.54 4.54 4.54 4.54
SRT-Temperature Interactions: TSS Components - Linear A x Linear B - Linear A x Quadratic B - Quadratic A x Linear B - Quadratic A x Quadratic B -**Remainder	13.50 3.95 2.30 3.15 1.81 2.19	8 1 1 1,	1.69 3.95 2.30 3.15 1.81 0.55	3.52 8.27 4.94 6.58 3.77 1.15	2.64 4.54 4.54 4.54 4.54 3.06
SSC or TSC Components - Linear A x Linear B - Linear A x Quadratic B - Quadratic A x Linear B - Quadratic A x Quadratic B - **Remainder	4.82 2.05 0.59 1.18 0.46 0.54	8 1 1 1 1	0.60 2.05 0.59 1.18 0.46 0.14	10.04 34.17 9.83 19.67 7.67 2.33	2.64 4.54 4.54 4.54 4.54 3.06
Pure Error (PE):	7.18	15	0.48	· •	
SSC or TSC	6)86	15	0.06		-
Note: *values x 10 <sup>3</sup> ** Remainder = Cubic B x Lin Cubic B x Qua Quartie B x L Quartic B x Q	dratic A inear A	•		,	•

result was used in preparation of Table E3. The pure error sum of squares was determined from repeats (Himmelblau, 1970). In determining ANOVA results for the pooled models (Table E4 and E5) the pure error was estimated by a pooled sum of squares based on individual pure error values and related degrees of freedom.

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TABLE E3 ANOVA FOR REPARAMETERIZED ARRHENIUS MODEL FOR COMBINED SLUDGE UNIT RATE RESULTS 10 DAY SRT

Source of Variation	*Sum of Square (SS)	Degrees of Freedom	*Mean Square (MS)	MSLOF MSPE	F α= .95
(Reparameterized Mod	el: K= 0.0308	3 e -12450/R (計	- <del>1</del> 0))		
After Regression Pure Error (PE) Lack of Fit (LOF)	0.397 0.293 0.104	9 5 4	0.044 0.059 0.026	0.45	5.19
Note: * values x 10	3			,	

TABLE E4 ANOVA FOR REPARAMETERIZED ARRHENIUS MODEL FOR COMBINED SLUDGE UNIT RATE RESULTS 4, 7, AND 10 DAY SRT WITH K/K\*

Source of Variation	Sum of Squares (SS)	Degrees of Freedom .	Mean Square (MS)	MSLOF MSPE	F a:=.95	F = .99
(Reparameterized M	odel: K/K* = e	17600/R ( <del>1</del>	1 T <sub>0</sub> ))			,
After Regression	11.639	53 32	0.220 0.017			,

TABLE E5 ANOVA FOR REPARAMATERIZED ARRHENIUS MODEL FOR COMBINED SLUDGE FILTERABLE TKN REMOVAL RESULTS 4, 7, AND 10 DAY SRT WITH K/K\*

Source of Variation	Sum of Squares (SS)	Degrees of Freedom	Mean Square (MS)	MSLOF MSPE	F α;=.95	F α =.99
(Reparameterized M	lodel: K/K* = e	15400/R (1 -	1 <sub>T0</sub> ))		,	
After Regression Pure Error (PE) Lack of Fit (LOF)	2.865 0.342 2.523	<sup>-</sup> 53 32 21	0.054 0.011 0.120			
		_,	<del></del>	10.92	1.9	2-5

E

```
PPOGRAM SUTTON
     DIMENSION X (25), Y (26), SCRAT (300)
     DIMENSION TH(2), SIGNS(2), DIFF(2)
     COMMON X
     EXTERNAL 10 DEL
   3 READ 4, NOB, TH(1), TH(2), JS
     IF (IFFOF (60). EQ.-1) STOP
   4 FORMAT(I3,F10.3,F10.3,I3)
     DO 10 I=1,NOS
     READ 1000,X(I),Y(I)
1000 FORMAT (7X,F7.5,F7.4)
  10 PRINT 1000, X(I), Y(I)
     PRINT 4, (NOB, TH(1), TH(2))
     PRINT 6, JS
   5 FORMAT (20X, 144PROBLEM NUMBER, I3)
     SIGNS(1) = 1
     SIGNS(2)=1
     DIFF(1) = .01
     DIFF(2)=.01
     EPS1=1.0E-7
     EPS2=1.0E-7
     CALL UWHAUS (1, MODEL, NOB, Y, 2, TH, DIFF, SIGNS, EPS1, EPS2,
    610,.01,1Q.,SCPAT)
     GO TO 3
     END
    SUBROUTINE MODEL (NPROB, TH, F, NOB, NP)
    COMMON X
    DIMENSION TH(1), F(1), X(26)
    DO 10 I=1.NOB
 10 F(I)=TH(2)*EXP(TH(1)*X(I))
    RETURN
    END
    SUBROUTINE UWHAUS (NPROB, MODEL, NOB, Y, NP, TH, DIFF, SIGNS, EPS1, EPS2,
   1 MIT, FLAM, FNU, SCRAT)
    DIMENSION SCRAT(1)
    IA=1
    IR=IA+NP
    IC=IB+NP
    ID=IC+NP
    IE=ID+NP
    IF=IE+NP
    IG=IF+NOB
    IH=IG+NOB
    II = IH + NP * NOB
    IJ = IH.
                HAUSS9 (NPROB, MODEL, NOB, Y, NP, TH, DIFF, SIGNS, EPS1, EPS2, MIT
         CALL
       Plam, FNU, SCRAT(IA), SCRAT(IB), SCRAT(IC), SCRAT(ID),
   2 SCRAT(IE), SCRAT(IF), SCRAT(IG), SCRAT(IH), SCRAT(II),
      SCRAT(IJ)
   RETURN
   END .
```

```
SUBROUTINE HAUSSO (MPRBO, MODEL, NOO, Y, NO, TH, DIFZ, SIGNS, EP1S, -P5,
     1MIT, FLAM, FNU, G.P., E, PHI, TB, F, R, A, D, OEL 7)
                    FORTRAN II VERSION
C
                    4. J. WERTZ
      ADAPTED FOR THE CDC 6+00 (J. F. MAGGREGOR)
r
      DIMENSION TH(NQ), DIFZ(NO), SIGNS(NO), Y(NBO)
      DIMENSION Q(NQ), P(NQ), F(YQ), PHI(YQ), TB(YQ)
C
      DIMINSION F (NBO), R(NBO)
C
      DIMENSION A (NO, NO), D(NQ, NQ), DELZ(NBO, NO)
      DIMENSION TH(1), DIFZ(1), SIGNS(1), Y(1), Q(1), P(1), \tilde{\epsilon}(1),
         PHI(1), T8(1), F(1), R(1), A(1), D(1), DELZ(1)
      ACOS(X) = ATAN(SQRT(1.0/X**2 - 1.0))
      CN = AN
      NPROB = NPRBO
      NOB .= N80
      EPS1 = EP1S
      EPS2 = EP2S
      NPSQ = NP + NP
       NSCRAC = 5*NP+YPSQ +2*NOB+NP-NO3
       PRINT 1000, NPROS, NOS, NP, NSCRAC
      PRINT 1001
       CALL GASSED(1, NP, TH, TEMP, TMEP)
      PRINT 1302
       CALL GASS60(1, NP, DIFZ, TEMP, TEMP)
      IF(MINJ(NP-1,50-NP,NOR-NP,MIT-1,999-MIT)) 99,15,15
      IF (FNU-1.0) 99, 99, 16
 16
      CONTINUE
      DO 19 I=1,NP
      TFMP = ABS(DIFZ(I))
      IF (AMIN1(1.0-TEMP, ABS(TH(I)))) 59, 39, 19
 19
      CONTINUE
      GA = FLAM
      NIT = 1
      LAOS = 0
      IF (EPS1) 5,70,70
      EPS1 = 0
   70 SSQ = 9
      CALL MODEL (NPROB, TH, F, NOB, NP)
      00 90 I = 1, NO9
      R(I) = Y(I) - F(I)
   90 SSQ=SSQ+R(I)*R(I)
      PRINT 1003, SSQ
C
                                                    BEGIN ITERATION
C
 100
      GA = GA / FNU
      INTCNT = 0
      PRINT 100,4, NIT
      JS = 1 - NOB
 101
      DO 136 J=1, NP
      TEMP = TH(J)
```

```
P(J) = \bigcap_{i=1}^{n} P(J) \times \bigcap_{i=1}^{n} P(J)
      (U)^{Q}+(U)HT=(U)HT
      Q(J) = C
      BCM + SU = SU
      CALL MODEL (MPRO), TH, DELTISI, NOR, NPL
      IJ = JS-1
      DO 120 I = 1, NOS
      IJ = IJ + 1
      DELZ(IJ) = DELZ(IJ) - F(I)
      O(J) = O(J) + O(J) + O(J)
120
      J(J) = J(J) \setminus S(J)
                                          Q=XT-R (STEEPEST DESCENT)
 130 \text{ TH}(J) = TEMP
      IF (LAOS) 131,131,414
     00 15ù I = 1, No
    ,00 151 J=1,I
      SUM = 3
      KJ = NO9 + (J-1)
      KI = NOS + (I-1)
      00 \ 160 \ K = 1, NOB
      KI = KI + 1
      KJ = KJ + 1
      SUM = SUM + DELZ(KI) * DELZ(KJ)
160
      TEMP= SUM/(P(I)+P(J))
      JI = J + NP+(I-1)
      D(JI) = TEMP
      I_{i}J = I + NP*(J-1)
      D(IJ) = TEMP
151
      F(I) = SORT(D(JI))
150
      CONTINUE
666
      DO 153 I = 1, NP
      IJ = I - NP
      DO 153 J=1,I
      IJ = IJ + NP
      \Delta(IJ) = D(IJ) / (E(I) + E(J))
      JI = J + NP*(I-1)
      A(JI) = A(IJ)
153
                                           A= SCALED MOMENT MATRIX
      II = - NP
      00 155 I=1,NP
      P(I)=Q(I)/E(I)
      PHI(I) = P(I)
      II = NP' + 1 + II
155
      A(II) = A(II) + GA
C
       CALL MATIN(A, NP. P., I. DET)
                                           P/E = CORRECTION VECTOR
C
       STEP=1.8
      SUM1 = 0 . .
      SUM2=0.
      SUM3=0.
```

	SUBROUTINE GASSED(ITYPE, NO. A. B. C)
	DIMENSION A (NO), B (NQ), C(NO, NQ)
	NP = NQ
	NP = NP/10
	LOW = 1
	LUP = 10
10	IF ( NR ) 15, 20, 39
15	RE TUR 1
20	LUP=NP
	IF (LOW .GT. LUP) PETURN
30	PRINT 500, (J.J=LOW,LUP)
	GO TO (40,60,80), ITYPE
+C	PPINT 600, (A(J), J=LOW, LUP)
•	GO TO 130
<u>0 č</u>	PRINT 600, (B(J), J=LOW, LUP) -
	GO TO 40 .
30	DO 30 I=LOW, LUP
90	PRINT 720,I,(C(J,I),J=LOW,I)
<del></del>	LOW2=LUP+1
	IF(LOW2 .GT. NP) GO TO 190
	DO 95 I=LOW2,NP
95	PRINT 720,I,(C(J,I),J=LOH,LUP)
100	LOW = LOW + 10
	LUP = LUP + 10
	NR = NR - 1
	GO TO 10
500	FORMAT(/18,9112)
600	FORMAT (10E12.4)
720	FCRMAT (1H0, I3, 1X, F7, 4, 9F12, 4)
1	CONTINUE
	RETURN
	END

NON-LIMEAR ESTIMATION, PROBLEM	NUMBER :	COMBINED SLU	DGE SSRT = 10 DAYS	
11 033ERVATIONS, 2 PAPAME	reas	58 SC	RATCH REQUIPED	
		1		
INITIAL PARAMETER VALUES				
1 2	······································	<del>/</del>		
-0.7000E+04 0.7000E+02	· · · · · · · · · · · · · · · · · · ·		,	
PROPORTIONS USED IN CALCULATING	) DIFF-RFI	VCE QUOTIENT	S	
1 2 0.1000E-01 0.1000E-01		·		
THATTAL CHA OF COMADES - C 07	· · · · · · ·			
INITIAL SUM OF SQUARES = 5.977		·		•
				<del> </del>
		ITERATI	ON NO. 1	
DETERMINANT' = 0.3935E+00	ANGLE IN		RD. =41.57DEGREE	<u> </u>
TEST POINT PARAMETER VALUES		4		
-0.3981E+04 0.3069E-01				
TEST POINT SUM OF SQUARES = 0.	101.5-02			<del></del>
TEST FOINT SON OF SQUARCS = 00	1014			
PARAMETER VALUES VIA REGRESSION	J			•
	·		1	
1 2 -0.3981E+04 0.3069E-01			1	<del></del>
				<del></del>
LAHBDA = 0.1005-02			SUM OF SO	QUARES
	Ai	FTER REGRESSIO	N = 0.1013855 E-02	
				·
,		ITERATI		
DETERMINANT = 0.7160E+00	ANGLE IN	N SCALED COO	RD. =25.330EGREE	ES .
TEST POINT PARAMETER VALUES				<del>"</del>
-0.6333E+04 0.3133E-01			,	<u> </u>
TEST POINT SUM OF SQUARES = 0.	4066E-03			<del></del>
[2	<b>\</b>		•	
PARAMETER VALUES VIA REGRESSION	N .			
1 2 -04.6333E+04 0.3133E-01		,		
15	• • •	<del></del>	•	
LAMBDA = 0.100E-03	· · · · · · · · · · · · · · · · · · ·		SUM OF SO	QUARES
	AFT	ER REGRESSION	= 0.4065500E-03	

DETERMINANT = 0.4524E+63	ITERATION NO. 3 ANGLE IN SCALED COORD. =13.830FGREES
TEST POINT PARAMETER VALUES -0.6258E+04 0.3076E-01	
TEST POINT, SUM OF SQUAPES =	0.3 16 5 = - 0 3
PARAMETER VALUES VIA REGRESS	ION
1 2 -0.62585+94 0.30765-01	
LAMBOA = 0.1905-34	SUM OF SQUARES AFTER REGRESSION = 0.3968095 E-03
DETERMINANT = 0.4596E+GG	ITERATION NO. 4  ANGLE IN SCALED COORD. =45.39DEGREES
TEST POINT PARAMETER VALUES -0.6254E+04 0.3076E-01	ANGEL IN SCALLO GOOKO. 245. 390 COKELS
TEST POINT SUM OF SQUARES =	0.39635-03
PARAMETER VALUES VIA REGRESS	ION
1 2 -0.6254E+04 0.3076E-01	
LAMBDA = 0.100E-05	SUM OF SQUARES AFTER REGRESSION = 0.3968077 E-03
DETERMINANT = 0.4600E+00	ITERATION NO. 5 ANGLE IN SCALED COORD. =43.27 DEGREES
TEST POINT PARAMETER VALUES -0.6254E+04 0.3076E-01	<u> </u>
TEST POINT SUM OF SQUARES =	0.39635-03
PLARAMETER VALUES VIA REGRESS	I ON
4 1 2 -0.62545+04 0.3076E-01	
LAMBDA = 0.100E-06	SUM OF SQUARES  AFTER REGRESSION = 0.3968077F-03

	CEUJAV NC				
0.4477=-01	0,21145-01	0.3075E-01	0.6515E-01	0.13545-01	0.13045-01
0.2114 E-01	0.2395 E-01	0.3076E-01	0.4477 E-01	0.6516E-01	
ESIQUALS			**************************************		
C-1367E-C1	0.1063E-02	0.30388-02	C.5942E-02	0.757CE-03	<u>-0.21435-02</u>
-0.9371 E-03	0.8446E-02	-0.5662E-02	0.6829 E-02	-0.9371 E-03	<del></del>
ORRELATION	MATRIX		,		
1	2		1		
1 1.0000				<del></del>	
2 0.7348	1.0000	<del></del>	1		****
	<u> </u>				
<del></del>					
0RMALIZING 1 0.1212E+06	ELEMENTS 2 0.3560E+00				
1 0.1212E+06	2 0.3560E+00	0.4409E-04,		OF FREEDOM	
1 0.1212E+06 ARIANCE OF	2 0.3560E+00 RESIOUALS =	· · · · · · · · · · · · · · · · · · ·			HYPOTHESIS)
1 0.1212E+06 ARIANCE OF NDIVIDUAL C	2 0.3560E+00 RESIDUALS = CONFIDENCE LI	· · · · · · · · · · · · · · · · · · ·	9 DEGREES		HYPOTHESIS)
1 0.1212E+06 ARIANCE OF	2 0.3560E+00 RESIDUALS = CONFIDENCE LI	· · · · · · · · · · · · · · · · · · ·	9 DEGREES		HYPOTHESIS)
1 0.1212E+06 ARIANCE OF NDIVIDUAL C	2 0.3560E+00 RESIDUALS = CONFIDENCE LI 2 .0.3549E-01	· · · · · · · · · · · · · · · · · · ·	9 DEGREES		
1 0.1212E+06 ARIANCE OF NDIVIDUAL C	2 0.3560E+00 RESIDUALS = CONFIDENCE LI 2 .0.3549E-01	· · · · · · · · · · · · · · · · · · ·	9 DEGREES		HYPOTHESIS)
1 0.1212E+06 ARIANCE OF NDIVIDUAL O 1 0.4645E+04 0.7864E+04	2 0.3560E+00 RESIDUALS = CONFIDENCE LI 2 0.3549E-01 G.2603E-01	MITS FOR EAC	9 DEGREES	ON LINEAR H	HYPOTHESIS)
1 0.1212E+06 ARIANCE OF NDIVIDUAL O 1 0.4645E+04 0.7864E+04	2 0.3560E+00 RESIDUALS = CONFIDENCE LI 2 0.3549E-01 G.2603E-01	MITS FOR EAC	9 DEGREES H PARAMETER  CH FUNCTION  0.7376E-01	ON LINEAR H	
1 0.1212E+06 ARIANCE OF NDIVIDUAL O 1 0.4645E+04 0.7864E+04	2 0.3560E+00 RESIDUALS = CONFIDENCE LI 2 0.3549E-01 G.2603E-01	MITS FOR EAC	9 DEGREES H PARAMETER  CH FUNCTION 0.7376E-01	VALUE 0.1825E-01	0.1825E-01

```
IF (NIT - MIT) 100, 100, 280
  2700 PFINT 2710
  2710 FORMAT (//115HJ*** THE SUM OF SQUARES CANNOT BE REDUCED TO THE SUM
      10F SQUARES AT THE END OF THE LAST ITERATION - ITERATING STOPS
 C
                                                   END ITERATION
  280
       PRINT 1311
       PPINT 2001, (F(I), I = 1, 408)
       PRINT 1012
       PRINT 2001, (R(I), I = 1, NOB)
       SS 0=SU49
       IOF=NOB-NP
       PRINT 1315
       I = 0
        CALL MATIN(D, NP, P, I, DET)
       DO 7692 I=1,NP
       II = I + NP^*(I-1)
7692 E(I) = SQRT(D(II))
       00 340 I=1,NP
       JI = I + NP+(I-1) - 1
       IJ = I + NP*(I-2)
       00 340 J = I, NP
       JI = JI + 1
       A(JI) = D(JI) / (E(I) + E(J))
       IJ = IJ + NP
  340
        A(IJ) = A(JI)
        CALL GASS60(3, NP, TEMP, TEMP, A)
       PRINT 1016
        CALL GASS60(1; NP, E, TEMP, TEMP)
       IF(IDF) 341, 410, 341
  341
       SDEV = SSQ / IDF
       PRINT 1014, SDEV, IDF
        SDEV = SQRT(SDEV)
       0.0 391 I=1,NP
       P(I)=TH(I)+2.0*5(I)*SDEV
       TB(I) = TH(I) - 2.0 + E(I) + SOEV
 391
       PRINT 1039
        CALL GASS60(2, NP, TB, P, TEMP)
       LAOS = 1
       GO TO .101
  414
       00 415 K = 1, NOB
       TEMP = 0
       00 420 I=1.NP
       DO 420 J=1, NP
       ISUB = K+NOB^*(I-1)
       DEBUG1 = DELZ(ISUE)
 C
       DEBUG1 = DELZ(K + NO8*(I-1))
       ISUB = K+NOB*(J-1)
       DEBUG2 = DELZ(ISUB)
       DEBUG2 = DELZ(K + NOB*(J-1))
 C
       IJ = I + NP*(J-1)
```

```
DO 231 I=1.4P
      SUM1 = P(I) * PHI(I) + SUM1 ·
      SUM2 = P(I) + P(I) + SUM2
      SUM3 = PHI(I) + PHI(I) + SU 43
      PHI(I) = P(I)
 231
      TEMP = SUM1/SQRT (SUM2*SUM3)
      TEMP = AMIN1(TEMP, 1.0)
      TEMP = 57.235 ACOS(TEMP)
      PRINT 1041, DET, TEMP
 170
      00 220 I = 1, N^2
      P(I) = PHI(I) *STEP / F(I)
      Te(I) = TH(I) + P(I)
      CONTINUE
 22C
      PRINT 7000
 7000 FORMAT (30HO TEST POINT PAPA TETER VALUES )
      PRINT 2006, (TB(I), I = 1, NP)
      DO 221 I = 1, NP
      IF (SIGNS(I)) 221, 221, 222
 222
      IF(SIGN(1.0,TH(I))*SIGN(1.0,TB(I))) 563, 221, 221
 221
      CONT INUE
      SUMB=0
      CALL MOJEL(NPROB, T3, F, NOB, NP)
      DO 230 I=1, NO9
      R(I)=Y(I)-F(I)
      SUMB=SUMB+R(I)*R(I)
230
      PRINT 1043, SUM3
      IF(SUMB - (1.0+EPS1)*SSQ) 602, 662, 663
      IF ( AMIN1 (TEMP-30.0, GA)) 665, 665, 664
563
665
      STEP=STEP/2.0
      INTCNT = INTCNT + 1
      IF (INTCNT - 36) 170, 2700, 2700
  664 GA=GA+FNU
     INTENT = INTENT + 1^4
      IF (INTCNT - 36) 666, 2700, 2700
     PRINT 1007
662
      DO 669 I=1, NP
669
     TH(I) = TB(I)
      CALL GASSBO(1, NP, TH, TEMP, TEMP)
     PRINT 1040, GA, SUMB
      IF (EPS2) 229,229,225
 223 IF (EPS1) 270,270,265
     DO 240 I = 1, NP
225
      IF (ABS (P(I))/(1.E-20+ABS (TH(I)))-EPS2) 240, 240, 241
 241 IF (EPS1) 270,270,265
 240 CONTINUE
     PRINT 1009, EPS2
     GO TO 280
265
     IF (ABS (SUMB - SSQ) - EPS1*SSQ) 266, 266, 270
266
     PRINT 10:10, EPS1
     GO TO 280
270
     SSQ=SUMB
     NIT=NIT+1
```

```
DSBUG3 = O(IJ)/(DIFZ(I)*TH(I)*DIFZ(J)*TH(J))
     TEMP = TEMP + DEBUG1 > DEBUG2 > DEBUG3
      TEMP = 2.0 *SQRT (TEMP) * SDEV
     R(K) = F(K) + T \in MP
     F(K)=F(K)-TEMP
415
     PRINT 1308
     IE=0
     DO +25 I=1, NO3,16
     IE=IE+13
     IF (NOR-IE) 430,435,435
 430 IE=NO3
     PRINT 2001, (R(J), J = I, IE)
425
     PRINT 2005, (F(J), J = I, IE)
     PRINT 1933, NPROB
     RE TURN
99
     PRINT 1034
     GO TO 410
10000FORMAT (3841 NON-LINEAR ESTIMATION, PROBLEM NUMBER
                                                         I3,// I5,
    1 · 14H OBSERVATIONS, I5, 11H PARAMETERS | I14, 17H SCRATCH REQUIRED)
1001 FORMAT (/25HOINITIAL PARAMETER VALUES )
1002 FORMAT (/54HDPROPORTIONS USED IN CALCULATING DIFFERENCE QUOTIENTS )
1003 FORMAT (/25HOINITIAL SUM OF SQUARES =
                                            E12.4)
1004 FORMAT (/////45X.13HITERATION NO.
1007 FORMAT (/32HOPARAMETER VALUES VIA REGRESSION )
1008 FORMATICAMS4H0APPROXIMATE CONFIDENCE LIMITS FOR EACH FUNCTION VAL
    1UE
10090FORMAT (/62H0ITERATION STOPS - RELATIVE CHANGE IN EACH PARAMETER LE
    1SS THAN
               E12.4)
10100FORMAT (/62HCITERATION STOPS - RELATIVE CHANGE IN SUM OF SQUARES LE
    1SS THAN
              E12.4)
1011 FORMAT (22H1FINAL FUNCTION VALUES )
1012 FORMAT (////10HORESIDUALS
1014 FORMAT (Y/24HO VARIANCE OF RESIDUALS = ,E12.4,1H,14,
    120H DEGREES OF FREEDOM
1015 FORMAT(////19HOCORRELATION MATRIX
1016 FORMAT (////21HJNORMALIZING ELEMENTS
1033 FORMAT (//19HDEND OF PROBLEM NO.
1034 FORMAT (/16HOPARAMETER ERROR
10390FORMAT(/71H0INDIVIDUAL CONFIDENCE LIMITS FOR EACH PARAMETER (ON LI
    INEAR HYPOTHESIS)
                       )
10400FORMAT(/9H0LAMBDA = £10.3,40x,33HSUM OF SQUARES AFTER REGRESSION =
    1E15.7)
1041 FORMAT (14H DETERMINANT = E12.+, 6X, 25H ANGLE IN SCALED COORD. =
    1 F5.2, 8HDEGREES
1043 FORMAT (28HD TEST POINT SUM OF SQUARES =
                                                E12.4)
2001 FORMAT (/10E12.4)
2006 FORMAT (10E12.4)
    END
```

	SUDDONITHE MATTILLA NIVAD D ND DETA
	SUBPOUTINE MATIN(A, NVAR, B, NB, DET) DIMENSION A(NVAR, 1), B(NVAR, 1)
	COMMON/GASPAR/DUMIES(7), PIVOTM PIVOTH = A(1,1)
	•
	DO 550 ICOL = 1, NVAR
	PIVOT = A(ICOL, ICOL) PIVOTY = AMIN1(PIVOT, PIVOTY)
	·
C	OET = PIVOT * DET
. •	TIMES DINGT ON LOW DINGT SLEWENT
C	THEMEST TOVIC TOVIC YOU TOVICE TOVICE TOVICE TOVICE
t	A / TOOL TOOL ) - 4 0
	A(ICOL, ICOL) = 1.0
	DINOT - ANAVAIDINOT 4 5-23)
	PIVOT = AMAX1(PIVOT, 1.E-23) PIVOT = A(ICOL, ICOL)/PIVOT
	DO 350 L=1,NVAR
350-	A(ICOL, L) = A(ICOL, L) *PIVOT
350.	IF (NB . EQ. 0) GO TO 371
	Q0 370 L=1,NB
370	8(ICOL, L) = 8(ICOL, L)*PIVOT
C C	B(1000, E/ - B(1000, E/ FIVO)
C	REDUCE NON-PIVOT ROWS
<u> </u>	REDUCE ADM-FIVOT ROWS
371	DO 550 L1=1,NVAR
	IF(L1 .EQ. ICOL) GO TO 550
	T = A(L1, LCOL)
	A(L1, ICOL) = 0.
	DO 450 L=1,NVAR
* 450	A(L1, L) = A(L1, L) - A(ICOL, L) + T
450	IF(N8 :EQ. 0) GO TO 550
	DO 500 L=1,NB
500	B(L1, L) = B(L1, L) - B(ICOL, L) + T
	CONTINUE
770	RETURN
<del></del>	END
	with the control of t

## Development of Transfer Function-Noise Models

A number of transfer function-noise models (TF-N) describing the effluent filterable TKN and NO3-N from the combined and separate sludge systems (Tables Fl and F2) were developed according to the iterative procedure described previously—using the results from experiment D5. The computer programs involved in the development of model number C1 (Table F1) including calculation of the cross correlations and impulse response weights (Table F4), estimation of the parameters of the tentatively identified transfer function model (Table F5), and the re-estimating of the parameters of the combined TF-N model (Table F6) are presented as examples. In determining the most appropriate models to represent the results the methods included statistical comparison techniques and individual evaluation through diagnostic checking.

The comparison techniques involved extra sums of squares testing (Draper and Smith, 1968) in which more elaborate models containing a greater number of parameters were compared to simpler forms and the significance of the extra parameters assessed. Individual diagnostic evaluation involved assessment of the autocorrelation function of the residuals and cross correlations between the residuals and the input variables. Assuming the form of the TF-N model was correct and that the true parameter values are known, then the estimated 1 autocorrelation function of the residuals would be uncorrelated and distributed normally about zero with variance  $n^{-1}$ , where  $\tilde{n}$  is the number of data points in the series. A further assessment of the residuals involves taking the first k autocorrelations and computing the 0 statistic (Table F3). Comparing the result to the chi-square distribution  $(x^2)$  with k-p-q degrees of freedom, where p+q is the total number of parameters of the noise model, determines the significance of the residuals. In a similar manner the significance of the cross correlations between the residuals and the stationary input series for each variable can be computed by comparing the S statistic to the  $\chi^2$  distribution (Table F3)

TRANSFER FUNCTION - NOISE MODELS FOR COMBINED SLUDGE SYSTEM TABLE FI

Output Variable	Model No.	Model	R e Sum of Squares	sidual Degrees of Freedom	s Mean Square
Filterable TKN	5	Yt = 0.011 ± 0.004 ° X1 1-0.786 ± 0.1068 X1 <sub>t-1</sub>	292.4	116	2.52
	, 25	$^{+}$ 1-1.066 $\pm$ 0.1778 $^{+}$ 0.336 $\pm$ 0.1758 <sup>2</sup> at $^{+}$ 0.012 $\pm$ 0.004 $^{+}$ $^{-}$ $^{-}$ 1-0.731 $\pm$ 0.1318 $^{+}$ $^{+}$ $^{-}$ 1	327.4	117	2.80
	ຼິຍ	$^{+}$ 1-0.796 $\pm$ 0.111g at $Y_t = (0.151 \pm 0.065 + 0.148 \pm 0.065g) X_{2t-1}$	308.5	. 911	2.67
		+ 1.395 $\pm$ 0.957 $x_{t-2}$ + 1.0.857 $\pm$ 0.097 at			
N03-N	<b>5</b>	$Y_{t} = 0.051 \pm 0.075  \text{x}_{2t-3}$ - 0.003 \pm 0.002 \qquad \text{X}_{3t-3}	383.0	116	3.30
	် မှ	$Y_{t} = \frac{0.052 \pm 0.0838}{1 + 0.115 \pm 1.058} X_{2_{t-3}}$	382.9	715	3.33

Output Variable	Model No.	Model	R e Sum of Squares	Residuals Sum of Degrees of Squares Freedom	s Mean Square
	. ,	$\begin{array}{c} 0.003 \pm 0.002 \\ 1-0.737 \pm 0.3198 \end{array} \text{ X}_{3_{5-3}} \\ + \frac{1}{1-0.889 \pm 0.0838} \text{ at} \end{array}$			
	ce Yt	$\gamma_t = \frac{-0.0036 \pm 0.0022}{1-0.650 \pm 0.2688} X_{3_{t-3}}$	396.03	117	3.39
	:	+ 1-0.883 ± 0.0828 at			
where X <sub>1</sub> = X <sub>2</sub> = X <sub>2</sub> = X <sub>3</sub>	where X <sub>1</sub> = TKN (filterable) X <sub>2</sub> = Influent TKN (fil X <sub>3</sub> = OC (filterable)	$X_1$ = TKN (filterable) loading (g/day), $X_2$ = Influent TKN (filterable) concentration (mg/l), and $X_3$ = 0C (filterable) loading.	٠ کيد		

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#ABLE F2 TRANSFER FUNCTION - NOISE MODELS FOR SEPARATE SLUDGE SYSTEM

,	Output Variable	Model No.	Model	Sum of ' Squares	Resic Degrees of Freedom	Residuals of Mean m Square
	Filterable TKN	ls	Yt = 0.012 ±0.003 + 0.008 ±0.003 B X <sub>1</sub> t-1	90.5	115	0.79
•		SS	$^{1}$ 1-1.394 ±0.158 B + 0.540 ±0.157 B <sup>2</sup> at $^{1}$ 0.090 ±0.040 + 0.111 ±0.047 B $^{1}$ $^{2}$ $^{2}$ $^{2}$ $^{2}$ $^{2}$	118.9	114	1.04
			+ 0.502 ±0.548 X4t-2 1 -1.380 ±0.159 B + 0.543 ±0.157 BZ at			
	, NO3-N	S3	$Y_t = 0.049 \pm 0.057$ $X_{2_t-3}$ 0.004 $\pm 0.002$ $X_{3_{t-1}}$	210.9	115	7.83
•		\$	lan lan lan	210.8	114	1.85
			1 + 1-1.129 ±0.184 & + 0.222 ±0.182 & at			

TABLE F2 (Cont'd)

		,		Residuals	uals
Output Variable	No.	i anou	Sum of Squares	Degrees of Freedom	Mean Square
•	S <sub>5</sub>	$Y_t = \frac{-0.003 \pm .002}{1-0.822 \pm 0.160 \text{ g}} \text{ X}_{3_{t-1}}$	225.2	116	1.94
		+ 1-1.200 ±0.175 g + 0.280 ±0.177 g <sup>z</sup> at	:		
where	where X <sub>1</sub> = TKN (filterable) X <sub>2</sub> = Influent TKN (fi X <sub>3</sub> = OC (filterable)	$X_1$ = TKN (filterable) loading (g/day), $X_2$ = Influent TKN (filterable) concentration (mg/l), and $X_3$ = 0C (filterable) loading.	·	,	

TABLE F3. DIAGNOSTIC CHECKING OF FINAL EFFLUENT TKN AND NO3-N MODELS

	Output Variable	Model No.	Residual Autoco Q	Autoco Q	$\begin{array}{l} \text{rrelation Results} \\ \frac{\chi^2}{\alpha} = .95 \end{array}$	Cross Sax <sub>1</sub>	. Correl x : =.95	Cross Correlation Results $\frac{x^2}{\alpha}$ $\frac{x^2}{\alpha}$ $\frac{x^2}{\alpha}$ $\frac{x^2}{\alpha}$ =.95	ults x <sup>2</sup> =,95	Sax <sub>3</sub> a	x <sup>2</sup> =,95	
	F11terable TKN	. S?		16.82	22.36 22.36	9.93	22.36					
	NOS-N	\$3	<i>!</i>	12.26	22.36			14.07	25.00	15:45	23.68	
	· .	ਹੈਂ	,	9.79	23.68			12.47	25.00	8.03	23.68	
	where Q =	Q = 7, 15 K=1	z raą (k),									
	# •>	= n 15	rax (k),		,					,		
	2 Z d	≖ estima	$r^2$ = estimate of cross correl		ation function,					٠		
	n ros	mode.] r(	= model residuals,									
•	ii′ -≯	k ≕ lag,	V.				,					
	; is	series	n = series length, and									
	iί <b>☆</b>	$x = (1 - \beta)X$	` *									
											***************************************	1

CROSS CCRRELATIONS AND V WEIGHTS FOR SSC SYSTEM FILTERABLE TKN LOADING VERSUS EFFLUENT FILTERABLE TKN

```
DIMENSION A(24,126), X(120), Y(120), Z(120), W(126)
        CALL FORMS (1)
         IDENTIFY PARAMETERS
        X = FLOW(IGAL/MIN)

Y = FICTERABLE RAW FEED TKN(MG/L)

W = FILTERABLE EFFLUENT TKN(MG/L)

Z = LOADING (G1/DAY)

AW = MEAN FILTERABLE EFFLUENT TKN(MG/L)

AZ = MEAN LOADING (GM/DAY)

NOB = NUMBER OF LAGS
        NL = NUMBER OF LAGS
    DATA NOB/12C/, NL/20/

READ 1, ((A(I,J), I=1,8), J=1,120)

READ 1, ((A(I,J), I=9,16), J=1,120)

READ 1, ((A(I,J), I=17,24), J=1,120)

1 FORMAT(17X,F3.6,19X,F6.0,4X,F4.0,F5.0,F5.0,F5.0,2X,F5.0)
     3 READ 4, (IX, IY, IA)
     4 FORMAT(312)
        PRINT 4, IX, IY, IW
IELIEFOE (60) . EO. -1) STOP
        PRINT 1
  12 FORMAT(28X,1CHFILTERABLE,31X,1GHFILTERABLE)
  11 FORMAT(10x,14HFLOW(IGAL/MIN),3x,12HRF TKN(MG/L),5x, 115HLOADING(GM/DAY),8x,13HEFF TKN(MG/L))
     9 FORMAT(16X,1HX,16X,1HY,19X,1HZ,19X,1HW)
         SW=0.0
        SZ=0.0

OC 5 K=1,NOB

X(K)=A(IX,K)

Y(K)=A(IY,K)

W(K)=A(IW,K)
        Z(K)=(X(K)*Y(K))*6.552
SZ=SZ+Z(K)
SW=SW+W(K)
  PRINT 10, (X(K), Y(K), Z(K), W(K))
10 FORMAT(9X, F1G. 2, 7X, F10. 2, 10X, F1G. 2, 1GX, F1G. 2)
        CONTINU
        AW=SW/FLOAT(NOB)
AZ=SZ/FLOAT(NOB)
PRINT 101
101 FORMAT (///, 24X, 5HZ MEAN, 15X, 6HW MEAN)
PRINT 102.AZ, AW
PRINT 102.AZ, AW
102 FORMAT (/,2CX,F10.3,10X,F10.3)

00'81 K=1,N0B

H(K)=H(K)-AH

81 Z(K)=Z(K)-AZ

CALL IDENT TF(Z,W,NOB,NL,1)

GO TO 3

END
```

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```
SUBROUTINE IDENT TF(X,Y,NOB,NL,NDIFF)
IDENTIFICATION OF THE IMPULSE RESPON
               INVERSION OF MATRIX
IS SUBROUTINE REQUIRES THE SSPLIA
LIB MUST DE ATTACHED
          SSPLIB MUST
        COMMON SDA, SDX
DIMENSION X(1), Y(1), AC(50), GAM(20,23), VGAM(400)
DIMENSION CCITZI), CCZ(ZI), L(4JC), M(4ZC), V(ZJ), VN(ZU)
        NDATA=NOB
        ND = 1
        PRINT 18
  18 FORMAT (52H1 CROSS CORRELATIONS AT + AND - LAGS FOR NDIFF = 6)
-6 CALL CROSS(X,Y,NO3,NL/661,602)
CALL ACORR(X,AC,SDZ, NOB,NL)
BUILDING THE MATRIX OF INPUT AUTOCORRELATION GAM
        BUILDING THE
        00 10 J=1,NLL
        GAM(J.J)=1.0
II=NL-J+1
00 20 I=2,II
        <del>I H= I + J-1</del>
        GAM(IW, J) = AC (I-1)
CONTINUE
        GAM (NE, NE) = 1.0
        DO 33 J=2,NL
  DO -0 I=1,JJ
40 GAH(I,J)=GAH(J,I)
       CONTINUE
PRINT 42, ((GAM(I,J),I=1,NL),J=1,NL)
FORMAT(10X,20F6.2)
TRANSFORMING GAM TO A VECTOR MATRIX VGAM
DO 50 J=1,20
DO 60 I=1,25
        IR=NL+(J-1)+I
VGAM(IR)=GAM(I,J)
        CONTINU
           INVERSION OF MATRIX
FINV AND GMPRO ARE 18M SCIENTIFIC SUBROUTINES
        N=NL*NL
CALL MINV(VGAM,NL,D,L,M)
        PRINT 51.0
       FCRMAT(//////,10x,44 D =,E2J.8)

NLL=NL*NL

PRINT 42, (VGAM(I), I=1, NLL)

CALCULATION OF THE TRANSFER FUNCTION PARAMETERS

CALL GASOU3(VGAM,CC2,V,NL,NL,1)

CALL GASOU3(VGAM,CC1,VN,NL,NL,1)
  OO 16 I=1.NL
V(I)=V(I)*SDA/SDX
16 VN(I)=VN(I)*SDA/SDX
PLOTTING PARAMETERS
       PRINT 8, ND
FORMAT(///.16x, 38H V WEIGHTS AT + AND - LAGS FOR NDIFF=,12)
CL=SQRT(VGAM(1)/FLOAT(NOB))*SDA*2.0/SDX
        CLN=-CL
        \mathbf{n}\mathbf{n}
        T=FLOAT(K)
TT=-T
        <del>KK=K-1</del>
        K1=-KK .
       PRINT 4, K1, VN(K), KK, V(K)

FORMAT (5X, 13, 5X, F6, 3, 10X, 13, 5X, F6, 3)

PRINT 12, CL
        FORMATIZZ, 53H APPROX.
                                                   95 PER CENT CONFILMIT ON IMPULSE RESPONSE
       1.F10.3)
NDATA=NDATA-1
        NO=NO+1
IF(NO-GT.NDIFF)GO TO 100
DO 41 I=1,NDATA
  41 Y(1) = Y(1/1) = Y(1/1)

PRINT 19
        FORMAT (52H1
                                     CROSS CORRELATIONS AT + AND - LAGS FOR NDIFF = 1)
        RETURN
<del>-100</del>
         END
```

```
SUBROUTINE ACORR(Z,AC,SDZ,N,NL)
     DIMENSION Z(1), AC(1)
     NL1 = NL+1
     T\hat{N} = N
     5-7-
     DO 13 I=1, N
SZ = SZ+Z(I)
ZBAR = SZ/IN
13
     00 10 JJ=1,NL1
     SZZ = J.
     NN = N-J
     00-11-I=1,NY-K=I+J
     $ZZ=$ZZ+(Z(I)-ZBAR)*(Z(K)-ZBAR)
ACTJJ1 = $ZZZZTN
     SOZ = SORT (AC(1))
            ACLL
00 12 J=1,NL
12 AC(J) = AC(J+1)/VZ
     END
    SUBROUTINE CROSS(X,A,NOB,NL,CC1,CC2)
    COMMON SDA, SDX
DIMENSION X(NOB), A(NOB)
DIMENSION CC1(41), CC2(41)
CC1 ARE CROSSCORRELATIONS AT NEGATIVE LAGS
CC2 ARE CROSSCORRELATION AT POSITIVE LAGS
CALL CROORP(X, A, CC2, SDX, SDA, NOB, NL)
    CALL CROORR (A, X, CCI, SDA, SDX, NOB, NL)
PRINT 6
                       CROSS-CORRELATIONS BETWEEN MANIPULATED VAPIABLES X(T) + A(T+K), //)
    FORMAT (79H
   1RESIDUALS
    NL1 = NL+1
DO 7 K=1+NL1
    KK = K-1
K1 = -KK
    PRINT 8, KI,CC1(K),KK,CC2(K)
FORMAT (5X,I3,5X,F6.3,10X,I3,5X,F6.3)
CL = 2.0/SQRI(FLOAT(NOB))
    PRINT 12 , CL FORMAT(//,55H APPROX. 95 PERCENT CONF. LIMIT ON CROSS-CORRELATIONS
 PRINT 9, SDX, SOA
9 FORMAT(/, 28H STANDARD DEVIATIONS
                                                          S(X) = F12.4,5X,6HS(A) = E12.4
    DO 10 J=1, NL1
    0 = 0 +
                                    CC2(J)*CC2(J)
<u> 11</u>
    Q = Q*FLOAT(NO3)
NDF = NL+1
PRINT 11, Q,NDF
11 FORMAT(/,25H CHI SQUARED STATISTIC = ,F5.2,/,1CH BASED ON(,12,46H
1NO. OF DYNAMIC PARAMETERS) DEGREES OF FREEDOM)
    RETURN
    END
    SUBROUTINE CRCORR(X,Y,CC,SDX,SDY,N,NL)
DIMENSION X(N),Y(N),CC(1)
    SX = 0.
SY = C.
        = [ •
    ŠŸŸ = Ō.
    DO 2 I=1,N
    SX = SX+X(1)
SY = SY+Y(I)
SXX = SXX+X(I)*X(I)
              SYY+Y(I)*Y(I)
    SYY =
     TN = N
    SDY = SORT((SXX-SX*SX/TN)/TN)
SDY = SORT((SYY-SY*SY/TN)/TN)
    NL1
          = NL+1
         3 K=1,NL1
    סס
    SXY = 0.
         = N-K+1
    NN
    DO 4 I=1.NN
KK = I+K-1
             CC(K) = (SXY/TN)/(SDX*SDY)
    RETURN
```

FLOW(IGAL/MIN)	FILTERABLE RF TKN(MG/L)	LOADING(GM/DAY)	FILTERABLE SEF TKN(4G/L)
X • • • • • • • • • • • • • • • • • • •	01 0001 0001 0001 00000000000000000000	55336458204279348728141039264756976666553530 43776122439381489859162633337074546867537379 5178331469757384631928825716947051477531416 45643677641191198742287398946880209652819987 111111111113322221122112411111111111111	10010010000000000000000000000000000000
00000000000000000000000000000000000000	980500000000000000000000000000000000000	542716293141728211620224354503815 982697697203962532858656829797803 322786824669889596557855473760700 8394247432335776032468765939571697	96000000000000000000000000000000000000

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5.854

CROSS CORRELATIONS AT + AND - LAGS FOR NDIFF = 0 CROSS-CORRELATIONS BETWEEN MANIPULATED VARIABLES + RESIDUALS  $X(T)*A(T+\zeta)$ 

.183	645+01	Σ
RELATIONS = 0	S(A) = 0.4564E+01	= 316.48 NAMIC PARAMETERS) DEGREES OF FREEDOM
ON CROSS-COR	95.8213	AMETERS) DEGR
95 PERCENT CONF. LIMIT ON GROSS-CORRELATIONS = 0.183	DEVIATIONS S(X) =	RED STATISTIC = 316.48
APPROX. 95 PERC	STANDARD DEVIAT	CHI SQUARED STA BASED ON (21 NO.

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APPROX. 95 PER CENT CONF LIMIT ON IMPULSE RESPONSE = 0.014

CROSS CORRELATIONS AT + AND - LAGS FOR NDIFF = 1 CROSS-CORRELATIONS BETWEEN MANIPULATED VARIABLES + RESIDUALS .X(T)\*A(T+K)

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0.024
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-19
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APPROX. 95 PERCENT CONF. LIMIT ON CROSS-CORRELATIONS = 0.183 STANDARD DEVIATIONS S(X) = 68.2263 S(A) = 0.1961E+01 CHI SQUARED STATISTIC = 45.79.

CHI SQUARED STATISTIC = 45.79.
BASED ON(21 NO. OF DYNAMIC PARAMETERS) DEGREES OF FREEDOM

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APPROX. 95 PER CENT CONF LIMIT OIL IMPULSE RESPONSE = 0.006

STOP

マROGRA 1 SUTTON
OIM-NSION SCRAT(1 100) , T等(8) ,A(2+,120) COMMON Z(123),W(120),X(123),Y(126) EXTERNAL MODELA
TPANSFER FUNCTION MODEL PARAMETER DETERMINATION FOR SSC SYSTEM ÉFFLUENT T AS A FUNCTION OF FILTERABLE TKN LOADING
IDENTIFY PARAMETERS  X = FLON(IGAL/4IN)  Y = FILTERABLE RAW FEED TKN(MG/L)  W = FILTERABLE EFFLUENT TKN(MG/L)  Z = LCA)ING (GM/DAY)
Z = CCADING (GM/DAY)  AW = MEAN FILTERABLE FEELUENT IKN(MG/L)  AZ = MEAN LOADING (GM/DAY)  NCB = NUMBER OF DATA POINTS  TH = CONSTANT PARAMETERS USED IN TRANSFER FUNCTION MODEL  NP = NUMBER OF TH VALUES
PROGRAM IDENTICAL TO TABLE F6 EXCEPT FOR FOLLOWING SUBROUTINE AND OUTPUT
SUBROUTINE MODELA (NPROB,TH,A,NOB,NP) DIMENSION TH(8),A(1) COMMON Z(120),W(120),X(120),Y(120)
TRANSFER FUNCTION MODEL A
A(1)=0.0 D0 100 I=2,N03 180 A(I)=(TH(1)*A(I-1))+W(I) 1-(TH(1)+W(I-1)) 1-(TH(2)*Z(I-1))
END
OUTPUT  NON-LINEAR ESTIMATION, PROBLEM NUMBER 1
120 OBSERVATIONS, 2 PARAMETERS 374 SCRATCH REQUIRED
INITIAL PARAMETER VALUES
- 0.7700E+30 - 0.1508E-01
INITIAL SUM OF SQUARES = 0.10115+04
ITERATION NO. 1
DETERMINANT = 0.4216E+00 ANGLE IN SCALED COORD. =24.89DEGREES  TEST POINT PARAMETER VALUES
0.7581E+00 0.1263E-01

```
TEST POINT SUM OF SOUAPES = 0.9292E+63
PARAMETER VALUES VIA REGRESSION
                                                                SUM OF SOUARES
-LAMBDA = 2.100=- )2
                                                 AFTER REGRESSION = 0.9291839E+03
                                                ITERATION NO.
                                   ANGLE IN SCALED COORD. =17.79DFGREES
                C.4146E+uS
DETERMINANT =
TEST POINT PARAMETER VALUES 0.75662+00 0.12602-01
TEST POINT SUM OF SQUARES =
                                0.9291E+C3
PARAMETER VALUES VIA REGRESSION
              0.1260E-01
 0.7566E+60
                                                                SUM CF SQUARES
LAMBDA = C.1065-03
                                                 AFTER REGRESSION = 0.9290996E+03
                                                 ITERATION NO.
                                   ANGLE IN SCALED COORD. =17.23DEGREES
DETERMINANT = 0.41335+00
TEST POINT PARAMETER VALUES 0.7564E+00 0.1261E-01
TEST POINT SUM OF SQUARES =
                                C. 9291E+03
PARAMETER VALUES VIA REGRESSION
                  . 2
 0.75645+00
              0.1261E-01
                                                                SUM OF SQUARES
LAMBDA = 0.100E-04
                                                AFTER REGRESSION = 0.9290978E+03
                    RELATIVE CHANGE IN EACH PARAMETER
                                                                      0.1900F-92
FINAL RESIDUAL VALUES
 0.0000E+00 -0.1215E+01 -0.1881E+01 -0.2468E+01 -0.3123E+01 -0.3001E+01
 -G.3Q3ZE+C1 -0.332ZE+01 -0.2777E+C1 -C.292CE+C1 -C.4C61E+01 -0.4u69E+C1
 -0.1788E+01 -0.2066E+01 -0.1325E+00/-0.886ZE+00 -0.3639E+00 -0.2287E+00
-0.1189E+61- 0.3081E+61-
                           8.3978E+61
                                        <del>-0+46755+01--0+54785+01--0+33826+01</del>
 <u>0.39785+01 -0.56575+00 -0.34625+61 -0.49025+01 -0.53895+01 -0.63525+61</u>
 -0.42695+01 -0.58015+00
                            0 \2554E+01
                                         C.4399E+01
                                                      0.4945E+01
         + 0 1.
                                    +00
                                                 -60
                                                               0-00-0
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-0.85312+20 0.76042+30	6 + 2 3 8 6 F + c 1	<del>6.22162+01</del>	-0.93785+00	<del>- 0 + 2651E+6+</del> -
-0.35995+00 -0.18145+01	-9.2360E+01	-0.40282+01	-6.26685+31	-0.1480E+01
-0.73012+00 -0.2848E-01	0.2787F+u3	U.1874E+05	0/10145-31	C.1525E+01
£.5943E+01	J.144(E+31	C.1080E+C1	C.5777E+30	[.2692±+5[
<del>0.31202+00 0.461+2+00</del>	-3.3365E-01	-0.7043E+GC	-0.1562E+91	-0.2350c+01
-0.3267E+01 -0.3100E+01	-0.35325+61	-6.2943E+C1	-3.1367E+01	-U.6144E+LL
-0.31925+00 0.99915+00	0.1-015+01	L.1952E+[1	0.2179E+ú1	0.2499E+01
0.2597E+01 0.3060E+01	0.22868+01	C.1775E+C1	G.3151E+01	0.3639E+61
6.4232E+01 6.1253E+01	-3.4095E+63	-E.2867E+61	-6.44532+)1	-0.3210E+ti
-C.458EE+E0 0.8867E+31	0.2024E+31	-G.3181E+01	-0.3186E+91	-C.2582E+01
-0.1901E+01 -0.2427E+01	-0.1393E+01	-C.7996E+00	0.3881E-31	0.9143E+00
0.16655+01 0.34145+01	0.3357E+C1	(.3964E+01	9.3623E+J1	€.3371E+61
0.3875E+61 0.2596E+81	0.32005+61	ۥ2923E+01	0.1568E+01	-0.1373E+06

CORRELATION MATRIX

1 1.3000

2 -0.7660

1.0000

NORMALIZING ELEMENTS

0.1145E-01 0.5161E-03

VARIANCE OF RESIDUALS = 0.7874E+01, 118 DEGREES OF FREEDOM

INDIVIOUAL CONFIDENCE LIMITS FOR EACH PARAMETER (ON LINEAR HYPOTHESIS)

1 0.82062+00 0.15512-01 0.69212+00 0.97165-02

AUTO AUD PARTIAL CARRELATIONS OF THE RECTON	2.11
NOTO NATIONAL PROPERTY OF THE 452100	460

Ι	AUTO:	PARTIAL
		•
1	€ 800	0.860
2	0+524 0+266	-0.321 -0.095
. 5 . 6	-6.084 -0.187	-0.069 -0.077
8 9	-0.210 -0.227 -0.217	0.067 -0.149
10 11 13	-0.184 -0.143	-0.007 -0.030
13	-0.107	-0.176

1 +	-0.1-3	-0.482	
15 10	-0.170 -0.171	-0.020	
18	-0.157 -0.103	-0.459	
19	υ•ύ[1 <del>6•1[1</del>		
APPROX. 95 PERC	ENT CONF. LIMIT CT	DORFELATIONS = 0.163	and the state of
CHI-SQUARED STA	<u>                                      </u>	RAMETERS) DEGREES OF FR	EE TON
SASEU UN 123 -	MO OF SINCHASITO	144": 1E421 NE34EE2 OF FR	

STOP

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PROGRAM SUTTON
          DIMENSION SCRAT(1000), TH(8), A(24,12))
COMMON Z(120), W(120), X(120), Y(120)
           EXTERNAL MODELA
 TPANSFER FUNCTION - NOISE MODEL PARAMETER DETERMINATION FOR SSC SYSTEM EFFLUENT TKN AS A FUNCTION OF FILTERABLE TKN LOADING
         IDENTIFY PARAMETERS

X = FLOW(IGAL/MIN)
Y = FILTERABLE RAW FEED TKN(MG/L)
W = FILTERABLE EFFLUENT TKN(MG/L)
Z = LOADING (GM/DAY)
AW = MEAN FILTERABLE FFFLUENT IKM
                                                               FEEL WENT IKM (15/L)
          AW = MEAN LOADING (GM/DAY)

AZ = MEAN LOADING (GM/DAY)

NOB = NUMBER OF DATA POINTS

TH = CONSTANT PARAMETERS USED IN TRANSFER FUNCTION NOISE MODEL

NP = NUMBER OF TH VALUES

NPROB = NUMBER OF DIFFERENCINGS
        DATA MIT, FLAM, FNU/1C, .c1, 10./
DATA EPS1, EPS2/1.CE-07, 1.DE-C3/
READ 1, ((A(I,J), I=1,8), J=1,120)
READ 1, ((A(I,J), I=9,16), J=1,120)
READ 1, ((A(I,J), I=17,24), J=1,120)
FORMAT(1/X,F3.0,19X;F6.0,4X,F4.0,F5.0,F5.0,F5.0,F5.0,2X,F5.0)
READ 4, IX, IY, IW, ICOUNT
IE(IECC, ICC), FG.-1) STOP
         FORMAT (412)
  PRINT 4, IX, IY, IW, ICOUNT

READ 8, WPROB, NOB, NP

8 FORMAT(315)
PRINT 5J, NPROB, NOB, NP

5U FURMAT(1UX, 315)
READ 2, (TH(I), I=1, NP)
2 FORMAT(E5, 2)
PRINT 12
           PRINT 12
   12 FORMAT(28X, 104FILTERABLE, 31X, 104FILTERABLE).
   11 FORMAT (10x,14HFLOW(IGAL/MIN),3X,12HRF TKN(MG/L),5X,
115HLOADING(GM/DAY),8X,13HEFF TKN(MG/L))
     9 FORMAT (16X,1HX,16X,1HY,19X,1HZ,19X,1HW)
           SH= u . C
          DO 5 K=1,NOB
X(K)=A(IX,K)
Y(K)=A(IY,K)
          W(K) = A(IW,K)
          Z(K) = (X(K)*Y(K))+6.552
SH=SH+N(K)
SZ=SZ+Z(K)
   PRINT 1), (X(K), Y(K), Z(K), W(K))
10 FORMAT(9X,F10.2,7X,F10.2,10X,F10.2)
          CONTINUE
AH=SW/FLOAT (NOB)
AZ=SZ/FLOAT (NOB)
A Z=SZ/FLUAT (NU5)

PRINT 202

202 FORMAT(///,24X,6HZ MEAN,15X,6HW MEAN)

PRINT 201,AZ,AW

201 FORMAT(/,2GX,F10.3,10X,F10.3)

00 81 K=1,N08

W(K)=W(K)-AW

81 Z(K)=Z(K)-AZ

IF(ICCUNT.EQ.1)GO TO 91

TECTROUNT.F0.2)GO TO 92
          IF(ICOUNT.EQ.2)GO TO 92
IF(ICOUNT.EQ.3)GO TO 93
IF(ICOUNT.EQ.4)GO TO 94
          IF (ICOUNT.EQ.5)GO
                                                     TO
                                                              95
           IF (ICCUNT.EQ.6) GO TO 96
                       <del>.0UNT。EQ.7160</del>
          IF (ICCUNT.EQ.8) GO TO 98.
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E NU

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IF (ICCUNT. FQ. 9) 30 TO 99 IE (ICCUNT. FQ. 13) SQ IO
   IF (ICOUNT.EQ. 11) GO TO 101
CALL ISHAUS (NPROB. MODELA, NOUS, NP, TH, EPS1, EPS2, MIT, FLAM, FNU, SCRAT)
   CALL
   CALL
SO TO
          TSHAUS (NPROB, MODELS, NOB, MP, TH, EPS1, EPS2, MIT, FLAM, FNU, SCRAT)
   CĂLL TŚHAUSTNP 209, MODELC, NOB, MP, TH, EPSI, EPSZ, MIT, FLAM, FN, U, SCRATT
GO TO 3
          <u>ISHAUS (NPRUB, 10DELD, NOB, NP, TH, EPS1, EPS2, MIT, EL AM, ENU, SCRAT)</u>
   GO TO
          TŠHAUS(NPRJB,40DELE,NUB,NP,TH,EPS1,EPS2,MIT,FLAM,FNU,SCRAT)
   CALL
   60
96 CALL TSHAUS (NPROB, MODELF, NOB, NP, TH, EPS1, EPS2, MIT, FLAM, FNU, SCRAT)
   GOTO
   CALL
GO TO
          TSHAUS (NPRIE, 1)JELG, NIU, NP, TH, EPSI, EPS2, MIT, FLAM, FNU, SCRAT)
          TSHAUS (NPROD, MODELH, NOB, NP, TH, EPS1, EPS2, MIT, ELAM, ENU, SCRAT)
   CALL
   GO TO
          TSHAUS(NPROB, MODELI, NOB, NP, TH, EPS1, EPS2, MIT, FLAM, FNU, SCRAT)
99 CALL
   CĂLL TSHAUS (NPROB, MODELJ, NOB, NP, TH, EPS1, EPS2, MIT, FLAM, FNU, SCRAT) GO TO 3
   CALL TS
          TSHAUSINPROB, MODELK, NOB, NP, TH, EPSI, EPS2, MIT, FLAM, FNU, SCRAT)
   SUBROUTINE MODELA (NPROB, TH, A, NOB, NP)
   COMMON Z (120), N (120), X (120), Y (120)
   TRANSFER FUNCTION NOISE MODEL A
   A(1) = G \cdot G
   A(2) = 0.0

A(3) = 0.0
   DO 100 I=4,NOB
A(I)=(TH(1)*A(I-1))+H(I)-(TH(3)*H(I-1))
  1+(TH(4)+W(1-2))
1-(TH(1)+W(1-1))
   + (TH(3) * TH(1) * W(T-2)
  1-(TH(4)*TH(1)*W(1-3))
1-(TH(2)*Z(I-1))
  <del>ュ+₹ŦĦ₹Ŝ}*ŦĤ₹₴Ĵ</del>*
  Î-(TH(4)+TH(2)+Z(Î-3))
RETURN
```

```
SUBROUTINE TSHAUS (NPROB, MODEL, NOB, NP, TH, EPS1, EPS2, MIT, FLAM, FNU,
 SCRATI
RESIDUALS ARE RETURNED IN SCRAT(IG) WHERE IG=5*NP+1
DIMENSION SCRAT(1)
TA=1
IB=IA+NP
IC=IB+NP
ID=IC+NP
IE = IO+NP
      IF+NP
IH=IG+NOB
II = IH + NP + NOB
      GALL HAUSTS (NPROB, MODEL, NOB, NP, TH, EP, FNU, SCRAT (IA), SCRAT (IB), SCRAT (IC), SCRAT (ID),
                                                                        EPS1, EPS2, MIT
,FLAM, FNU,
SCRATTIE),
RETURN
                                 SCRATTIGI,
                                               SCRATTIHI, SCRATTIII, SCRATTIJII
F.ND
```

. ·

```
SUBROUTINE HAUSTS (NPR30, MUDEL, NBO, N

1MII.FLAM.FNU. 0.2.F.PHI.TA. R.A.D.DEL

FORTRAN II VERSION

ADAPTED FOR THE CDC 6+00 (J. F. MACGREG
                                                                                             NO, TH,
                                                                                                                                  EPIS, EP 2S.
                                                                (J. F. MASSREGOR
          DIMENSION TH(MQ), R(MRO)
DIMENSION Q(MQ), P(MQ), F(MQ), PHI(MQ), TB(MQ)
DIMENSION A(MQ,MO), D(MQ,MQ), DELZ(MBC,MQ)
Q(1)
          DIMENSION TH(1),
                                                                                                    2(1),
                                                                                                               P(1), E(1),
          DIMENSION AC(16), PP(16)
ACOS(X) = ATAN(SCRT(1.0/X*+2 - 1.0))
                                                             P(1) . A(1) . D(1) .
                                                                                                    DELZ(1)
           <del>40---43</del>-
          VPROD = NPR30
NOB = VBO
EPSI = EPIS
          EPS1 = EPIS
EPS2 = EP2S
NPS0 = NP *
            NSCRAC = 5*NP+NPSQ + NOB+NP*NOB
PRINT 1000, NPROB, NOB, NP, NSCRAC
          CALL GASSEO(1, NP, TH, TEMP, TMEP)

IF(MIN3(NP-1,53-NP,NOB-NP,MIT-1,999-MIT))99,15,15

IF(FNU-1.0)99, 99, 16

CONTINUE
15
          0.019I=1
          IF( ABS(TH(I)) )
CONTINUE
                                                99,39,19
19
          GA = FLAM
          NIT = 1
IF(EPS1) 5,76,70
CPS1 = 0
SSQ = 0
CALL MODEL INPROS
    70
                    MODEL INPROS.
                                                TH. P. NOB. NP)
          DO 90 I = 1, NOB
SSQ=SSQ+R(I)**?(I)
PRINT 1003, SSQ
                                                                                                     BEGIN ITERATION
          GA = GA / FNU
100
          INICHT = 6
PRINT 1034,
JS = 1 - NOB
                                   NIT
101
         00 130 J=1,NP
TEMP = TH(J)
P(J) = 0.01*TH(J)
TH(J)= TH(J)+F(J)
          Q(U) = 0
         CALL MODEL (NPROB, TH, DELZ(JS), NOB, NP)

IJ = JS-1

00 120 I = 1, NOB

IJ = IJ + 1

DELZ(IJ) = R(I) - DELZ(IJ)

Q(J) = Q(J) + DELZ(IJ) + R(I)

Q(J) = Q(J)/P(J)
                                                                                Q=XI*R (SIFEPEST DESCENT)
         TH(J) = TEMP

DO 150 I = 1, NP

00 151 J=1,I

SUM = 0
 1.30
131
          KJ = NOB+(J-1)
                    NO3*(1-1)
         00 160 K = 1, NOB
         XJ = KJ + 1

XJ = KJ + 1

SUM = SUM + DELZ(KI) * DELZ(KJ)

TEMP= SUH/(P(I)*P(J))

JI = J + NP*(I+1)

D(JI) = TEMP
160.
         IJ = 1 + NP+(J-1)
D(IJ) = TEMP
E(I) = SORT(D(JI))
151
150
         CONTINUE : 1, NP
666
          00 153 J=1,I
```

IDF-NOO-NP

```
\frac{1}{1} = \frac{1}{1} + \frac{1}{1}
                                            / (F(I)*F(J1)
             JI = J + NP + (I-1)
             (UI)A = (IU)A
  153
                                                                                     -A= SCALED MOMENT MATRIX
             II = - VP
            70 155 I=1,NP
             PHI(I)=P(I)
                        N2 +
            A(II) = A(II) + GA
  155
     ---I-1
               CALL MATIN(A, MP, P, I, DET)
                                                                                       P/E = COPRECTION VECTOR
                SIEP=1.0
             SUM1=4.
             SUM2= C.
             SUM3=u.

JO 231 I=1,NP

<u>SUM1=P(I)*PHI(I)+SUM1</u>
            SUM1=P(I)*P(I)+SUM1
SUM2=P(I)*P(I)+SUM2
SUM3= PHI(I) * PHI(I) + SUM3
PHI(I) = P(I)
TEMP = SUM1/SQRT(SUM2*SUM3)
TEMP = AMINI(IEAP, 1.G)
TEMP = 57.295*ACOS(TEMP)
PRINT 1041, DET, TEMP
P(I) = PHI(I) *STEP / E(I)
TB(I) = TH(I) + P(I)
CONTINUE
231
 <del>-170</del>-
             CONTINUE
PRINT 7000
  220
             FORMAT (30HOTEST POINT PARAMETER VALUES PRINT 2006, (T3(I), I = 1, NP)
              SUMB=0
             SUMB=0
CALL MODEL(NPROS, TS, R, NOS, NP)
DO 230 I=1,NOS
SUMB=SUMB+R(I)*R(I)
PRINT 1043, SUMB
IF(SUMS - (1.0+EPS1)*SSQ) 652, 662, 663
IF( AMIN1(IEMP-30.0, GA)) 655, 665, 664
STEP=STEP/2.0
INTCNT = INTCNT + 1
INTCNT = 361 170. 2790. 2706
230
665
              IF (INTENT
                                        <del>35) 175, 2798, 2786</del>
    664 GA=GA*FNU

INTCNT = INTCNT + 1

IF (INTCNT - 36) 666

662 PRINT 1007
                                       36) 666, 2700, 2700
   662
             PRINT 100/

00 669 I=1.NP

TH(I)=T8(I)

CALL GASS60(1, NP, TH, TEMP, TEMP)

PRINT 1040, 6A, SUM9

IF (EPS2) 229,229,225

IF (EPS1) 270,276,265

DO 240 I = 1, NP

IF (A9S(P(I))/(1.E-20+ABS(TH(I)))-EPS2) 240, 240, 241

IF (EPS1) 276,270,265
669
  225
              CONTINUE
PRINT 1009, EPS2
GO TO 286
IF (ABS(SUMB - SSO) - EPS1*SSQ) 266, 266, 270
     240
   265
     266 PRINT 1010, EPS1

GO TO 280

270 SSQ=SUMB
              NIT=NIT+1
              IF(NIT-MIT) 100, 100, 280
   2700 PRINT 2710

2700 PRINT 2710

2710 FORNAT (77115HO**** THE SUM OF SQUARES CANNOT BE REDUCED TO THE SUM

10F SQUARES AT THE END OF THE LAST ITERATION - ITERATING STOPS /)
                                                                                                               END ITERATION
   280 PRINT 1011

PRINT 2001, (R(I), I = 1, NOB)

SSQ=SUMB
```

END

```
I = (
                 CALL
                              14TIN(D. NP. P. I. DET)
              00 7692 I=1,NP
II = I + NP*(I-1)
  7692 E(I) = SORT(O(II))

OC 346 I=1,NP

JI = I + NP*(I-1) - 1

IJ = I + NP*(I-2)

OO 346 J = I, NP
              A(JI) = D(JI + IP)
                            = D(JI) / (E(I)*E(J))
                 <del>(IL)A = (LI)A</del>
 <del>- 3+0</del>-
              CALL GASSEG(3, NP, TEMP, TEMP, A)
PRINT 1316
CALL GASSEG(1, NP, E, TEMP, TEMP)
IF(IDF) 3-1, 410, 3-1
SDEV = SSO / IDF
                                                             E, TEMP, TEMP)
   341
              PRINT 1314, SDEV,
SDEV = SQRT (SDEV)
00 391 I=1, NF
              P(I)=14(I)+2.0+E(I)*SDEV
TB(I)=TH(I)-2.0*E(I)*SDEV
3 31
              PRINT 1939

CALL GASS6G(2, NP, TB, P, TEMP)

CALL ACORR(R.AC.SDE.NO3.15)

CALL PARTAL(AC.PP,15)
              PRINT 59
   59 FORMAT (1H1,47H AUTO AND PARTIAL CORRELATIONS OF THE RESIDUALS,//
19X,1HI,14X,4HAUTO,14X,7HPARTIAL//)
DO 58 I=1,15
       58 PRINT 57 , 1, AC(T), PP(T)
57 FORMAT (8X, I2, 13X, F6. 3, 13X, F6. 3)
CL = 2.J/SORI(FLOAT(NOR))
             PRINT 57
       PRINT 54,CL
54 FORMAT(//5CH APPROX. 95 PERCENT CONF. LIMIT ON CORRELATIONS = ,F5.
            <del>13)</del>
              CHI =
              DO 56 I=1,15
      56 CHI = CHI+AC(I) +AC(I)

CHI = CHI+FLOAT(NOB)

PRINT 55. CHI

55 FCRMAT(/25H CHI-SQUARED STATISTIC = .F6.2/65H BASED ON (15 - NO. O
                  STOCHASTIC PARAMETERS) DEGREES OF FREEDOM /)
             CONTINUE
              RETURN
             PRINT 1034
  99
  99 PRINT 1334
GO TO 410
1000GFORMAT(38H1NON-LINEAR ESTIMATION, PROBLEM NUMBER I3,// I5,
1 14H 03SERVATIONS, IS, 11H PARAMETERS T14, 17H SCRATCY REQUIRED)
1003 FORMAT(/25H0INITIAL PARAMETER VALUES )
1003 FORMAT(/25H0INITIAL SUM OF SQUARES = E12.4)
1007 FORMAT(////55x,13HITERATION NO. I+)
1007 FORMAT(/32HJPARAMETER VALUES VIA REGRESSION )
13090FORMAT(/62HJITERATION STOPS - RELATIVE CHANGE IN EACH PARAMETER LE
                                                                                                              114. 17H SCRATCH REQUIRED)
  1SS THAN E12.4)
10100FORMAT (/52H0ITERATION STOPS - RELATIVE CHANGE IN SUM OF SQUARES LE
1SS THAN F12.4)
  1SS THAN F12.4)
1G11 FORMAT(22H1FINAL RESIDUAL VALUES )
1G12 FORMAT(///10H0RESIDUALS )
1G14 FORMAT(//24H6 VARIANGE OF RESIDUALS )
120H DEGREES OF FREEDOM )
1G15 FORMAT(///19H3CORRELATION MATRIX
1016 FORMAT(///21H3NORMALIZING ELEMENTS )
1034 FORMAT(//16H0PARAMETER ERROR )
                                                                         RESIDUALS =
                                                                                                             <del>y E 1 2 o 4 y 1 H y I 4 y</del>
  1039GEORMAT (/71 HG INDIVIDUAL CONFIDENCE LIMITS FOR FACH PARAMETER (ON LI
1NEAR HYPOTHESIS) )
1040CFORMAT (/9HOLAMBDA = E10.3,40%,33HSU4 OF SQUARES AFTER REGRESSION =
1041 FORMAT(14H DETERMINANT = E12.4, 6X, 25H ANGLE IN SCALED COORD. = 1 F5.2, 8HDEGREES )
1043 FORMAT(28HCTEST POINT SUM OF SQUARES = . E12.4)
2001 FORMAT(/10E12.4)
2005 FORMAT(10F12.4)
```

```
SUBROUTINE MATIN(A, NVAR, B, NB, DET)
        TIMENSION A(NVAR. 1), B(NVAR
COMMON/GASPAP/DU IES(7), PIV
PIVOTM = A(1,1)
        DO 550 ICOL = 1, NVAR
PIVOT = A(ICOL, ICOL)
PIVOTM = AMTRICPIVOT, PIVOTO
OET = PIVOT * DET
        DIVIDE PIVOT ROW BY PIVOT ELEMENT
        - 4 (ICCL + ICOL) = 1 + 6 ---
        PIVOT = AMAX1(PIVOT, 1.E-2J)
       PIVOT = ATTCOL, TCOLT/PIVOT

00 35L L=1,NVAR

A(ICCL, L) = A(ICOL, L)*PIVOT

IF(NB . EQ. G) GO TO 371

00 37 L=1,NB
35C
REDUCE NON-PIVOT ROWS
       DO 553 L1=1,NVAR

IF(L1 .50. ICOL) GO TO 550

T = A(L1, ICOL)

A(L1, ICOL) = 0.

30 450 L=1,NVAR

A(L1, L) = A(L1, L) - A(ICOL, L)+T

IF(NB .5Q. G) GO TO 550

DO 500 L=1,NB

B(L1, L) = B(L1, L) -B(ICOL, L)+T
371
                                                                               a
450
        B(L1, L
590
                    L) = B(L1, L) - B(ICOL, L) + T
 550
        RETURN
END
        SUBROUTINE GASSED (ITYPE, NQ, A, B, C)
          NP = NQ
          NR = NP/10 -
          LOW = 1
LUP = 10
        IF( NR )15,20,30
10
        RETURN
LUP=NP
15
20
        IF(LOW .GI. LUP) RETURN
PRINT 500, (J,J=LOW,LUP)
GO TO (40,60,80),ITYPE
PRINT 600 (A(J))J=LOW,LUP)
30
40
        GÒ TO 130
        PRINT 600, (B(J), J=LOH, LUP)
60
        GO TO 47
DO 90 I=LOW, LUP
PRINT 720. I. (C(J. I). J=LOW, I)
80
91
        LOW2=LUP+1
        IF (LOWY .GT. NP) GO TO 100
        PŘINT 720,1, (C(J,1), J=LOW, LUP)
95
          + k0J = W0J
+ 4UJ = 4UJ
100
                               10
          NR = NR - 1
          GO TO 10
        FORMAT(/IB,9112)
FORMAT(10E12.4)
50C
60 C
<del>720</del>
        <del>FORMAT(1116+13+1X+F7+4+9F12++)</del>
        CONTINUE
        RETURN
        ENU
       SUBROUTINE ACORR(Z,AC,SOZ,N,NL)
DIMENSION Z(1),AC(1)
       NL1 = NL+1
       TN = N
  DO 13 I=1,N
13 SZ=SZ+Z(I)
ZBAR=SZ/IN
       00 10 JJ=1,NL1
```

the state of the s

(3)

```
NN = N-J

DC 11 I=1*NN

K=I+J

11 SZ7=S7Z+(Z(I)-Z3AR)*(Z(K)-ZBAR)

10 AC(JJ) = SZZ/TN

SCZ = SORT(AC(1))

VZ = AC(1)

DO 12 J=1*,NL

12 AC(J) = AC(J+1)/VZ

RETURN
ENO

SUBROUTINE PARTAL (R, PAUTO, M)

DIMENSION P(1) PAUTO(1) PHAI(AB) PHAIN(AB)

PAUTO(1) = R(1)

PAUTO(1) = R(1)

PAUT(1) = P(1)*(1.-P(2))/(1.-P(1)**2)

PAUTO(2) = PHAI(2)

DAUTO(2) = PHAI(2)

DO 1 J=3*, M

L = I-1

FNUM = 3.

DENOM = 0.

DENOM = 0.

DENOM = DENOM+PHAI(J)*R(J)

PAUTO(1) = PHAIN(I)

DO 2 J=1*,L

Z PHAIN(J) = PHAIN(I)

DO 3 J=1*,I

3 PHAI(J) = PHAIN(J) - PHAIN(I)*PHAI*(K)

DO 3 J=1*,I

4 CONTINUE

RETURN
END
```

1 120 4	FTI TERARI F		ETLTERABLE
FLOW(IGAL/MIN)	RF TKN(MG/L) .	LOADING(GM/DAY) Z	EFF TKN(1G/L)
1.00 1.00	22.20 23.10 25.60	145.45 151.35 167.73	1.03 0.70
1.00 1.00	25.60	167.73 148.73	0.80
1.00	22.70 20.40 24.90	133.66 163.16	1.00 0.70 0.90
2.30 2.90	20.70 13.30	27.1 • 25 174 • 28	1.30 2.70
2.30 2.90 2.00 2.00 2.00	12.70 11.40	166.42 149.39 117.94	3,30 3,10 1,73
2.00 2.00 2.00	9.00 8.80 15.10	117.34 115.32 197.87	1.19 2.90 3.30
2.00	15.10 23.90 24.30	313.19 318.43	3.30 7.20
5 00 5 00 5 00	<b>₽</b> 22.50	294.84 286.98	8.63
2.00 2.00 1.00	20.90 21.60	273.87 141.52	10.10 11.70
1.10	21.60 21.60 35.10 33.90 28.70	229.98 222.11 188.04	12.27 13.10 13.77
1.00	27.20 35.50 37.60 14.30 14.60	178.21 232.60	14.00 -
2.30 2.00 2.00	37.60 14.30	495.33 187.39 191.32	12.30 10.90 7.20
2.00 2.00 2.00	11.20 12.90	146.75 169.04	4,93 2,63 1,63
2.00 1.00 1.00	16.40	184.77	3.40
1.00 1.00 1.00 1.00	18.40 16.10 13.90 9.90	120.56 135.49 91.07	3.40 5.90 8.33 9.40 9.20
1.00	9.90	64.86	9.43 9.20

1.60	6.50 17.50 14.60 14.60 14.60 13.30 25.40	57.66 127.76 95.00	3.00
1.30	÷9.50	127.76	5.6) 
1 10	14.50	45.40	
1.30 1.00 1.30 2.00 2.00 2.00	16.10	113,35 91,73 94,35 91,73	2.60 2.00 1.72 1.40 1.50
<del></del>			2 • U U
1.30	14.63	91.73	1 . 4 . 7
2.00	13.50	176.93	1.50
2 • 0 6	29.30	383.95 332.84	4.13
2.30	25.40	332.84	4.13 9.23
2 10		600.00	
2.36 2.00 2.00 1.00 1.30	18 37	2 <del>92,22</del> 247.67 328.91	<del>10.00</del>
2.00	25.10	328.91	10.51
1.00	72.40	146.76	111.4
1.10	12•ú0	7.8\$6.2	7.5j
1.33	22.32 18.33 25.10 22.40 12.40 6.54 5.30 4.00	146.76 78#62 42.59	10.83 10.53 9.90 10.43 7.53 5.03
1.00 1.00 1.00 1.00 1.00 1.00	5 • 3 9 4. 2 0	34.73	1.40
1.00	5.56	20.21 36.64	1.20
Ī.JČ.	6.00	39.31	1.03
1.30	6.00 9.00	50.97 ·	1.10
1.000	12.00	78.62	1.45 1.20 1.13 1.03 1.10 1.20
1.36	12.10	7.9.28	1.23
1.00	6-00	70 74	1.17
2.00	2.60	26. 21	∠•59 6 = 0
-2 - 9 6		45.86	2.50
2•30	5.00	65.52	2.50 6.50 2.59 1.42 1.20
2.00	6.70	87.80	ī.20
1.00 2.00 2.00 2.00 2.00 2.00 2.00	6.00 2.00 3.00 5.00 6.70 6.10 5.10	26.21 36.64 39.31 50.97 78.62 79.28 65.52 39.31 26.21 45.86 65.52 87.80 78.62 65.52 87.80 78.62 65.52	1,10 1,63 1,03
2.96	2 • CU 4 • 2 ft	65.5 <i>C</i>	1.63
2.00 2.00 2.00 2.00 2.00 2.00	8.10 12.00 14.80 12.00 13.50 16.90 25.50	174.83	1.00
2.00	12.60	157.25	1.00
2.00	14.89	157.25 193.94 157.25 176.90	4 / 0
2 - 10	12.00	157.25	1.80
1.00	13.50	1 46 9 9 0	1.80 1.50 1.50 1.00 1.00 1.00 2.20 4.00
1.00	25.50	110.73 167.08 190.01	1 • 2 J 1 : CO
_1_00	29.00	190.61	1.60
1.36 \ 1.00	5ۥ00	170.35 176.90 170.35 146.76	2.20
1.00	27.00	176.90	4.00
1.00	22 40	1/0.05	<del>5.60</del>
1.00	28.00		5.40
1.00	29.50	193.28	7 - 7 11
1.00	28.40	186.08	8.10
1 36	29.00	190.01	8-50
1.00 1.00 1.00 2.00	31.10	243.11 1.77.71	9.00
2.00	32.50	435.14 425.88	9.40
2,00	29.00 26.00 26.00 22.40 28.40 29.50 29.00 31.00 33.10 32.50 27.00	193.28 186.08 190.01 203.11 433.74 425.88 353.81 251.60	8.10 8.50 9.00 9.40 13.69 14.50
2.00	19.20	251.60	14.80
2.00 2.00 2.00 2.00 2.00 2.00	26.50 30.50 31.50 21.50 16.00	340 4 7 0	15.5J 16.03 18.40
2.00	30420 31.50	399.67 412.78	16.09
2.00 2.00 2.00 2.00 2.00 2.00	21.50	281.74	16.50
2.00	16.00	209.66 193.01	14.03
2.00	14.50	193401	16.50 14.03 10.03 7.00
2 • JU 2 . · 10	13.50 20.00	176.90	7.00
1.00	132111	262.08 85.18	/ • U IJ
1.00 1.00 1.30	13.00 12.40 11.90	81.24	9.93 17.10
1.30	11.00	81•24 77•97	17.10 8.60
1.00	10.50 12.00 16.23	58.8C	2.10 1.00 0.90
1.00	12.00		₹ • Ö.Ö
1.00 2.00 2.00	10.80	141,52	U-98
2.00	10.80 17.70	231.94	0.90 0.80
2.00	1,5.90	208.35	3.31
2.00 2.00 2.10	15.90 16.20 10.90	68.80 78.62 66.83 141.52 231.94 208.35 212.28	3.33 4.70
2.00	11.10	142.83	6.23
1.00	^ 22.70	148,73	<b>0 • / J</b> 7 - 20
1.00	11.10 ^ 22.70 18.70	145.45 148.73 122.52	6.70 7.20 8.83
			<del>-</del>

TABLE F6 (Con	t'd)		358
1.36 1.30	14.20 13.96	93.04 91.07	8 • 6 9 8 • 2 0
1.00	13.EU 11.60	39.11 76.00	7.33 6.61 5.81
2.30	15.90	136.28 208.35 205.73	5.73 7.5)
2.00 2.00 2.00	15.70 	<del></del>	<del>8 13</del> 7 • 23
2.50	9.88	123,18	5.03
<u>_</u>	MEAN	W MEAN	
	6.677	<del>- 5.834</del>	
NON-LINEAR ES			RATCH REQUIRED
0.76065+06	2 - <del>6:13902-01 -6:1</del>	3 3 <del>63E+61                                    </del>	,
INITIAL SUM	JF SQUARES = C.	29622+53	
		الله المراقع في المراقع ا	
DETERMINANT :	= C.2289E+00	ANGLE IN SCALED COO	ON NO. 1 RD. =37.190EGREES
TEST POINT PO	ARAMETER VALUES	156E+C1 0.3217E+00	
TEST POINT S	JM OF SQUARES =	0.29255+03	
PARAMETER VAL	LUES VIA REGRESS	ION	
1 0.781:5=+cc	2 0.10665-01 0.1	3 156F+01 [.3217F+00	
LAMBDA = 0 + 1	0.07-02	•	SUM OF SQUARES
- CRIBOR - 001	· · · · · · · · · · · · · · · · · · ·	^ AFTER REGRE	SSION = 0.2924856E+03
DETERMINANT	= 0.2120E+00	ITERATI ANGLE IN SCALED COO	ON NO. 2 RO. =49.000EGREES
TEST POINT P. 0.78592+03	ARAMETER VALUES 0.10662-01 0.1	365E+C1 C.3353E+00	
	UM OF SQUARES =	*	
PARAMÉTER VA	LUES VIA REGRESS	ION	
0.78995+00	0.1066E-01 0.1	3 065E+01 0.3353E+00	

استد ،

SUM OF SQUARES

· = |

```
TABLE F6 (Cont'd)
                                                                             359
                                                   FICH NUITARBII
                                     ANGLE IN SCALED COORD. =39.90DFGREES
 DETERMINANT = 0.21115400
 TEST PCINT PARAMETER VALUES
0.78632+01 0.10622-31 0.13605+01 0.33555+00
TEST POINT SUM OF SQUARES = 0.292+1+03
 PARAMETER VALUES VIA REGRESSION
                                           C.3355E+00
  0.7863E+00 0.1062E-01 0.1066E+01
                                                                   SUM OF SQUARES
 LAMBDA = 0.100 \Xi - 04
                                                  AFTER REGRESSION = 0.2924019E+03
                                                   <del>ITERATION NO</del>.
                                     ANGLE IN SCALED COORD. =49.56DEGREES
 DETERMINANT = 0.2109E+00
 TEST POINT PARAMETER VALUES
0.78642+00 0.1062E-01 0.1066E+01
                                           C.3358E+00
TEST PCINT SUM OF SQUARES = 0.23245+03
PARAMETER VALUES VIA REGRESSION
                              0.1365E+S1
                0.1062E-01
  0.78645+00
                                                                    SUM OF SQUARES
 LAMBDA = 0.100E-05
                                                  AFTER REGRESSION = 0.2924018E+03
 ITERATION STOPS - RELATIVE CHANGE IN EACH PARAMETER LESS THAN
```

FINAL RESIDUAL VALUES 0.0000E+00 -0.3706E+00 -0.7699E+00 -0.2332E+00 0.0000E+00 0.0000E+00 -0.6024E+00 -0.6801E+00 -0.1696E+00 -0.9410E+00 +0.1805E+01 -0.7151E+00 0 - 1 422E - 01 0.6878E+00 0.1771E+01 -C.1237E+01 6.1195E+01 -0.1365E+01 -G.1355<u>-</u>+61 - 6.1083<u>-</u>+11 0.2308E+01 -0.3504E+01 -0.1388E+01 -0.1516E+01 -0.2015E+01 -0.1625E+01 0.1666E+01 0.1367E+01 0.98665+00 G.6848E+00 0.4848E+00 0.16635+01 -0.4238E+00 -0.3872E-01 -0.9321E+00 -0.1885E+00 -0.1785E+00 -0.4555E+00 -0-1-19E+J1 -0-1852E+01--0-4655E-01 -0-3677E+00 0.2687E-01 <del>-0.76832-01</del> 0.6533E+3C +0.1747E+00 -0.3241E-01 -0.1518E+01 -0.6971E+00 -0.2296E+01 0.9772E-01 +0.4669E-01 +0.1996E+00 -0.1835E+00 0.1471E+01 -0.2055E+GG -0.1159E+30 -0.2428E-01 0.3020E+00 0.41836+01 -0.35336+01 0.7713E+00 ~E+4051E+00~ 0.74515+36 -8.12432+68 -0.1+682+88 -8.4113E+88 -0.1202E+01 -0.4768E+00 -0.1226E+01 -0.1405E+00 0.6105E+30 -0.9791E-01 0.3015E+00 0.8466E+00 0.5965E+00 0.8723E+00

4E+01

-0.9722E-01

•••••	360
0.72612+00 0.16372+31 0.16995+60 0.51225+00 0.20345+01	0.544CE+00
0.2335=+01 -0.1388=+01 -0.3344+10 -0.2068=+01 -0.1579=+01	0+5+00E+00
0.1587E+61	-[.39685+66
-0.3846E+00 -0.1263E+01 0.6343E+00 -0.4766E+00	0.52036+00
0.6919E+00 0.1934E+01 0.7385E+03 0.8957F+00 0.6124E+00	0.75592+00
- 6+ <del>5912=+03                                    </del>	<del>-,6 • 91115+9 C</del>
CORPELATION MATPIX	
2 - 2	
1 1.3000	
2 -0.6074 1.0000	•
3 0.1185 -0.1492 1.0000	
<del>4 0.1299 -6.0645 0.8013 1.8000</del>	
,	
NORMALIZING ELEMENTS	** * ** ** ** ** ** ** ** ** ** ** ** *
0.3334E-61 0.1239E-62 0.5590E-61 0.5527E-01	
0.3334E-61 0.1239E-62 0.5590E-61 0.5527E-01	
VARIANCE OF RECIPIALS - 1 25045. (4 444 250250 OF SPEEDON	*
VARIANCE OF RESIDUALS = G.25215+(1, 116 DEGREES OF FREEDOM	
INDIVIOUAL CONFIDENCE LIMITS FOR EACH PARAMETER (ON LINEAR H	YPOTHESIS)
1 /2 3 4	
0.8922±+00 0.1455=-01 0.1243E+C1 8.5113E+00 0.6805E+00 0.6683E-02 0.8883E+C0 0.1603E+00	
· · · · · · · · · · · · · · · · · · ·	**************************************
AUTO- AND PARTIAL CORRELATIONS OF THE RESIDUALS	1
- I AUTO PARTIAL	
	,
0.037	·
3 -0.008 -0.006 -0.0021 -0.022	
5 0.004 0.003 6 -0.177 -0.176	( /
7 0.043 0.034 8 -0.102 -0.091 9 -0.084 -0.097	
. 10 -0.034 -0.041	*
11 -0.103 -0.108 12 -0.160 -0.062 13 -0.032 -0.018	·
14	
15 -0.072 -0.111	1
APPROX. 95 PERCENT CONF. LIMIT ON CORRELATIONS = 0.183	- 4
CHI-SQUARED STATISTIC - 10.28	,
BASED ON (15 - NO. OF STOCHASTIC PARAMETERS) DEGREES OF FREED	UM

with k+1-(r+s+1) degrees of freedom where (r+s+1) is the number of parameters in the transfer function model. Applying these methods led to the final form of the TF-N models (Table 14).

## Effect of Altering Sampling Interval on TF-N Models

The optimal choice of the sampling interval is an important consideration when dealing with process control. In this regard McGregor (1976) has presented a procedure whereby given a discrete dynamic-stochastic model based on a given sampling interval, the alteration in the form and the parameters of the model can be determined for a new sampling interval. The approach is illustrated below for the linear dynamic-stochastic model developed to describe the effluent filterable TKN from the combined sludge system (Table 14) and leads to the new model form given in Table 15. A similar procedure was employed for determination of the new separate sludge effluent filterable TKN model.

Combined sludge effluent TKN TF-N model for new sampling interval

The original TF-N model was developed on the basis of a two hour sampling interval. In order to utilise the model for forecasting purposes, with respect to experiments D3 and D4, the form of the model and the parameters must be altered to account for a sampling interval of three hours. The stochastic portion of the original TF-N model is given by the autoregressive model

$$(1-1.066\beta + 0.336 \beta^2) N_t = a_t$$
 .....(50 with autoregressive parameters

$$\phi_{2hr}^{1} = 1.066$$
 $\phi_{2hr}^{1} = -0.336$ 

)./

by calculating the reciprocal of the roots of

 $(1-1.0668 + 0.336 \ \beta^2) = 0$  ..........(51) given by G1 and G2 the new parameter values can be determined from

$$\phi_{13hr} = G_{1}^{h}G_{2}^{h}$$
 .....(53)

where h is the number of times the basic sampling interval is increased (h = 3/2). By carrying out this procedure the new stochastic model was determined to be

$$(1-0.7254\beta + .1949 \beta^2) N_t = a_t$$
 .....(54)

The original transfer function model is also affected by the change in sampling interval. By assuming that the discrete transfer function model is a discretely coincident representation of an underlying continuous process of corresponding order, one can show using modified Z - transforms how the parameters of the model will change with sampling interval (Box and Jenkins, 1970 d). The original TF model is first order in nature and of the form

$$Y_{t} = \frac{0.001}{1 - 0.7868} X_{1_{t-1}}$$
 .....(55)

with parameters

$$\delta_{2hr} = 0.786$$
 $\omega_{2hr} = 0.011$ 

For the new sampling interval the parameter values can be determined from

$$\delta_{3hr} = \delta_{2hr}^{h}$$
 .....(56)

$$\omega_{3hr} = \frac{\omega_{2hr}(1-\delta_{2hr}^{h})}{1-\delta_{2hr}} \qquad .....(57)$$

These calculations lead to the new TF model given by

$$Y_{t} = \frac{0.0156}{1-0.697\beta} \quad X_{t-1} \qquad ......(58)$$

# Forecasting and Simulation Procedures Forecasting system response to impulse forcings

The forecasted eff]uent values in experiments D3 and D4 were determined by using the "difference" forms of the TF-N models (three-hour interval) together with the observed values of the input variable, filterable TKN loading  $(X_1)$ . Details of the computation methods are outlined elsewhere (Box and Jenkins, 1970C). To briefly illustrate the procedure reference will be made to experiment D3 and the combined sludge system results (Figure 26).

In order to forecast the initial effluent TKN results, past values of the influent variable  $(X_1)$  as well as the effluent TKN are necessary. By beginning the forecast following the baseline day of a the experiment these values were then available. The form of the TF-N model forecast equation also requires past values for the residuals  $(a_t$ 's), the difference between the observed and model results. These residuals were determined by fitting the TF-N model to the baseline day influent conditions and thereby determining model effluent values. Comparing the observed and model results allows determination of the residuals for the baseline day of the experiment. A computer program was written to carry out the above calculations and determine the forecast results (Table F7).

## Predicting responses to simulated input conditions

In order to utilize the transfer function (TF) and the TF-N models to predict the response of the combined and separate sludge systems to natural input conditions, the baseline day influent variations encountered in experiments D3 and D6 were simulated. Each simulation involved approximating the three-hour sampling results as a continuous record and then picking off discrete two-hour interval results. These results were used in conjunction with the TF and TF-N models (two-hour interval) to predict effluent results. As in the case with forecasting, the initial TF results are influenced by past values of the influent

```
TRANSFER FUNCTION - HOISE MODEL FORECASTING PROSPAI - RUN DO FILTERABLE EFFLUENT TKN , SSC SLUDGE SYSTEM
          OI FNSION A(5,50), X(50), Y(50), T(50)
CC4MON 7(50), W(50), TH(20), R(50)
CALL FORMS(1)
DATA NO3/17/, NAB/7/, NIB/11/
NU3=N48+2
           IDENTIFY INPUT
X = FLOW(GAL/MIN)
          Y = FILTERABLE RAW FEED TKN(MG/L)
W = FILTERABLE EFFLUENT TKN(MG/L)
NP = NUMBER OF TH VALUES
           R = TIME (HRS)
          READ 4.IX,IY,IW,NP,IR
FCRMAT(512)
PRINT.6.IX,IY,IW,NP,IR
          FORMAT (1H1,512)
           A7 = MEAN LOADING FROM EXPERIMENT D5 (GM/DAY)
AW = MEAN FILTERABLE EFFLUENT TKN FROM EXPERIMENT D5 (MG/L)
          READ 82,AZ,AW
F03MAT(16X,F7.3,2X,F5.3)
           PRINT
          FORMAT(////,19X,30HMEAN VALUES FROM EXPERIMENT D5,//)
          PRINT 86,AZ
FORMAT(20X,19HLOADING (GM/DAY) = ,F7.3,/)
PRINT 87,AW
FORMAT(20X,24 PRINT FOR THE COMMAT/20X PRINT FOR THE COMMAT/20X PRINT FOR THE COMMAT/20X PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT PRINT P
87 FCRMAT(26X,31HFILTERABLE EFFLUENT A (MG/L) = ,F5.3,///////
         READ 2, (TH(I), I=1, NP)
FCRMAT(F6.4)
PRINT 3
            READ IN MODEL PARAMETERS
   2
        FORMAT (3X, 28 HINPUT MODEL PARAMETER VALUES)
          90 98 J=1,NP
PRINT 97,TH(J),J
FORMAT(10X,F10.4,2X,3HTH(,12,1H))
98 CONTINUE
            READ IN DATA
           RFAD 1.((A(I,J),I=1,5),J=1,NOB)
FORMAT(3X,F4.1,2X,F3.1,F6.1,F6.1,F7.1)
PRINT 20
          FORMAT(///,8X,1HX,15X,1HY,14X,1HZ,14X,1HW,12X,1HR)
20
         PRINT 23
FORMAT(/,19X,10HFILTERABLE,22X,10HFILTERABLE)
       FCRMAI(/,19%,1000 1E,C.,222,12

PRINT 21

FCRMAT(2X,14HFLOW(IGAL/MIN),2X,12HRF TKN(MG/L),2X,

115HLOADING(GM/DAY),2X,13HEFF TKN(MG/L),2X,9HTIME(HRS))

GC 5 K=1,NOB

X(<)=A(IX,K)

Y(X)=A(IY,K)
           Y(X) = A(IY, K)

W(X) = A(IW, K)

R(X) = A(IR, K)

Z(X) = (X(K) *Y(K)) * E. 552

PRINT 10, (X(K), Y(K), Z(K), W(K), R(K))

FORMAT(7X, F4.2, 6X, F10.2, 5X, F10.2, 5X, F10.2, 4X, F10.2)
            CONTINUE
            DO 81 K=1,NOB
Z(K)=Z(K)-4Z
H(K)=W(K)-AN
            CONTINUE
            CALCULATION OF RESIDUALS FOR FIRST
                                                                                                                                                               VALUES: (24
                                                                                                                                                                                                          HR PERIOD)
            \dot{T}(1) = 3.0

T(2) = 0.0

T(3) = 0.0
```

```
ESI 11
      Englati///,6x,9HRESIDUALS,//)
      no 45 I=1, 7
print 80, I, T(I)
premat(5X, 24T(, I2, 1H), F5, 2)
      CCITINU
      Ť(I)=(TĤ(1)*Ť(I-1))+W(I)-(TH(2)*W(I-1))
    CONTINUE
      CALL FORECAST CALCULATION SUPROUTING FROT .
      CALL FROT (MCP, MIB, TO, MAB, AR)
      SUBROUTINE FROT (NOB, NIB, T, NAB, AW) .
      FCRECAST CALCULATION
      DIMENSION S (50), TA (50), TD (50), WT (50), WS (10) CCMMON Z (50), W(50), TH (20), P (50)
       \tilde{T} = 0
       DC 20
               J=NAB,NOB
       I=I+1
S(I)=Z(J)
  20 CONTINUE
       TA(1) = W(NAB)
      TA(2) = W(NAB+1)

TA(3) = W(NAB+2)
       PRINT 30
     FORMAT(/////,5x,45HYN = FORECASTED EFFLUENT FILTERABLE TKN VALUE
1,7,5x,36HY = OBSERVED EFFLUENT FILTERABLE TKN,7,5x,55HBARY = MEAN
2 EFFLUENT FILTERABLE TKN VALUE EXPERIMENT D5,7//////
  30
       PRINT
     FCRMAT(18x,8HFORE CAST,16x,8HQBSEPVED,/)
PRINT_13
  13 FORMAT(14X,2HYN,3X,7HYN-BARY,15X,6HY-BARY,8X,1HY,3X,9HTIME(HPS),/)
      LL=NAB+2
DO 11 L=NAB,LL
      WS(L)=W(L)+AV
PPINT 12, WS(L), W(L), WS(L), P(L)
FORMAT(12X, F6.2, 2X, F6.2, 16X, F6.2, 5X, F6.2, 5X, F4.1)
      CONTINUE
       K=NAB+2
       N=0
       FORECASTING NEXT 8 VALUES
     fic 101 J=4,NIB
TA(J)=(TH(2)*TA(J-1))-(TH(3)*TA(J-2))+(TH(1)*TA(J-1))
1-(TH(2)*TH(1)*TA(J-2))+(TH(3)*TH(1)*TA(J-3))
     1+(TH(4)*S(J-1))-(TH(2)*TH(4)*S(J-2))+(TH(3)*TH(4)*S(J-3))
       K='\+1
       IF(J. GT.4) GO TO IF(J. EQ. 4) GO TO
       CONTINUE
       TP (4) =TA (4) +T
       N=N+1
WT(K)=W(K)+AW
      TC(J) = TA(J) + AW
PRINT 6, TO(J) + TA(J) + N + W(K) + WT(K) + R(K)
FOR YAT(12X + F6 + 2 + 2X + F6 + 2 + 5X + 4 HT = 7 + 12 +
 100
                                                         ,12,5X,F6.2,5X,F6.2,5X,F4.1)
       RETURN
       ENT
           FINIS
X.LGO.
```

MEAN VALUES FROM EXPERIMENT 05

LOADING (GM/DAY) = 166.677
FILTERABLE EFFLUENT A (MG/L) = 5.850

INPUT MODEL PARAMETER VALUES

0.6970 TH( 1)

0.7254 TH( 2)

0.1950 TH( 3)

0.0156 TH( 4)

X	Y	Z	H	R
FLOW(IGAL/MIN) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 2.00 2	FILTERABLE RF TKN (MG/L) 15.70 15.70 10.20 7.10 7.80 4.10 17.20 17.50 11.50 9.80 9.80 9.80	LOADING (GM/DAY) 37.35 102.87 67.46 59.47 46.52 51.17 46.51 28.17 525.39 2203.15 60.97 60.92 36.04	FILTERABLE EFF TKN(MG/L) 1.70 1.00 1.20 1.10 1.10 0.90 1.30 0.90 3.70 4.30 1.20 1.00	TIME 36.000 000 000 000 000 000 000 000 000 00

#### RESIDUALS

T(1) 0.00 T(2) 0.00 T(3) 0.00 T(4) 0.25 T(5) 0.42 T(6) 0.32 T(7) 0.61 T(8) 0.27 T(9) 1.23

YN = FORECASTED EFFLUENT FILTERABLE TKN VALUE
Y = OBSERVED EFFLUENT FILTERABLE TKN
BARY = MEAN EFFLUENT FILTERABLE TKN VALUE EXPERIMENT D5

F	ORECAST			OBSERVED		
	YN-3 A			Y-BARY	Υ	TIME (HRS)
1.990 1.990 1.889 2.333 2.271 1.00	55550452845 	T = T = T = T = T = T =	12345678	55555555555555555555555555555555555555	1.19 1.3 1.9 3.7 43.1 0.7 0.7 1.0	0 24.00 24.00 247.00 227.00 336.00 336.00 45.00

variables and effluent results but in this case these values are unknown. This problem was overcome by assuming the influent variation over the 24 hour period was repeatable. This procedure, together with assuming initial values for the past effluent results, allowed the simulation to be carried out. By extending the computation for a number of repeatable influent sequences the results stabilized to the final predicted effluent sequence, independent of the initially assumed effluent values. In determining the TF-N model results, in addition to the requirement for past effluent and influent values a series of values for the white noise sequence  $(a_+'s)$  is required. This series was generated using a random numbers routine. The mean of the series was specified as zero and the standard deviation was set equal to the standard deviation of the residuals determined during the TF-N model development (experiment D5). The computer program developed to carry out the above computations and generate the stable predicted effluent results is illustrated in Table F8. for a particular example.

9

```
TIMENSION TY(103), TT(130), TX(130), KA(130), V(166), VX(130)
      TRANSFER FUNCTION - NOISE MODEL SIMPLATION PROGRAM

ASSELING OF IMPUT SONDITIONS
COMPUTATIONS FOR SSC SLUDGE SYSTEM - FILTERABLE EFFLUENT TKN (2 HR 40DFL
      (FIG. 35)
      IDENTIFY PARAMETERS

IY = PREDICTED FILTERABLE EFFLUENT IKN(MG/L) MINUS MEAN

VX = PESIDUAL RANDOM NUMBER ,08TAINED FROM G.C.I.W. COMPUTER LIBRARY
SUBROUTINE GAUSS. THIS SUBROUTINE COMPUTES A NORMALLY DISTRIBUTED RANDO ::
NUMBER AITH A GIVEN MEAN OF G.G AND A STANDARD DEVIATION OF 1.5877 ---

IX = PAN SEWASE FILTERABLE TKN LCADING(GM/DAY)

II = PREDICTED FILTERABLE EFFLUENT TKN

KA = HOUR OF PREDICTED FILTERABLE EFFLUENT TKN
               J=1,99
1, TX (J), \langle A(J)
       00 4
       READ
       <del>-PORMAT(2X,F8,3,2X,I4)</del>
CONTINUE
       PRINT 10
  1) FORMATTIOX, 42HRAW SEWAGE FILTERABLE TKN LOADING SEQUENCE, //)
00 12 J=1,12
PRINT 11,IX(J), <A(J)
       FORMAT(19X, F8.3, 2X, I4)
  12 CONTINUE
      00 102 1=1,97
READ 5,V(I)
FORMAT(2X,F5.2)
       CONTINUE
DO 101 I=1,97
VX(I+2)=V(I)
102
101 CONTINU
       FORMATITY///y10xy36HRANDOM NUMBER SEQUENCE FOR RESIDUALSy//)
       DO 13 J=3,14
PRINT 14,VX(J),J
      FORMAT(10X, F5. 2, 2X, 3HVX(, 12, 1H))
  13 CONTINUE
       PRINI
  16 FORMAT (////, 10x, 33HPREDICTED FILTERAGLE EFFLUENT TKN, //)
       INITIAL ESTIMATE OF PREDICTED FILTERABLE EFFLUENT(MG/L) MINUS MEAN
       TY(1)=3.05
TY(2)=2.65
       TY(3)=1.15
       DO 108 J=4,99
       TY(J) = VX(J) - (0.766 + VX(J-1)) + (1.366 + TY(J-1)) - (0.336 + TY(J-2))
      1+(0.785+1Y(J-1))-(1.066+0.786+1Y(J-2))+(0.336+0.786+1Y(J-3))
1+(0.911+TX(J-1))-(1.066+0.011+TX(J-2))+(0.336+0.011+TX(J-3))
        TT(J) = TY(J) + 5.85
100 CONTINUE
PRINT 19
       FORMAT(6X,3HTKN,5X,2HHR,8X,3HTKN,4X,2HHR//)
PRINT 18,(TT(J),KA(J),J=4,99)
FORMAT(2(F10.2,3X,14)/)
  18
       END
       RAW SEWAGE FILTERABLE TKN LOADING SEQUENCE
                          -25.677
                                            330
                                           500
700
900
                           34.677
```

Ł

-46.677

-81.677 32.0677

1100

36.323	<del>-1500-</del>
33.323	1700
23.323	1900
11.323	2100
13.323	2300
-3.323	-100

+ 4.4JOA	NUMBER S	EQUENCE F	UP RES	IDUALS
	<u>-vx(-3)</u>			
-1.52 -1.64	VX( -) VX( 5)			
	<del>√X ( -<u>ŷ</u> )</del>			· · · · · · · · · · · · · · · · · · ·
3 3 • + 6	VX (7) VX (8)			•
,-1,55	AX( 3)			
	VX (10)			
-1.54	VX (12)	•		
- ) • 30 <del>- 9 • 50</del>	VX (13) - <del>VX (14)</del>			
POEULO	TEO FILTE	SADIE SER	FLUSHT	TVN
-FKN-	— HR	IKN		
	•			
5.32	930	2.51	1165	
2.37	1300	3.22	1503	
7.91	1739	7.41	1900	
7.37	2105	7.08	230)	
6.11	100	5.12	300	
_4.24_	5.0.0	3.65	7.00	
3.10	900	1.23	1100	
1.76	1300	2.91	1500	
7.77	1700	7.36	1900	
7.34	2100	7.85	2366	
5.08	100	5.69	300	
4.22	500	3.63	700	
3.12	900	1.24	1100	
1.71	1300	2.92	1500	
7.78	1700	7.36	1900	<u></u>
7.34	2100	7.86	2300	
6.09	100	5.10	300	•
4.22	50C	3,63	700	
			· · · · · · · · · · · · · · · · · · ·	•
3.10	900	1,23	1110	
1.76	1300	2.91	1500	
7.77	1700	7.36	1900	· · · · · · · · · · · · · · · · · · ·
7.•34	2190	7.85	. 2300	<u>.</u>
6.08,	100	5.09	300	
•			,	•

5.00

#### APPENDIX G

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### Abbreviations and Symbols

Abbreviations and symbols appearing in this report, not commonly used or not defined within the report are listed on the following pages.

They are arranged according to where they appear in the report.

Abbreviations and symbols appearing in "Data Listing"

Appendix B:

OPERATING MODE	<pre>- reactor system under operation; 1 - SSC 2 - TSS 3 - TSC 4 - FSC</pre>
SSC	- single-stage combined sludge system
TSS	- two-stage separate sludge system
TSC	- two-stage combined sludge system
FSC	- five-stage combined sludge system
SSRT	- system solids retention time (days)
TEMP	- temperature ( <sup>O</sup> C)
SS	<ul><li>suspended solids (mg/l)</li></ul>
HACH NH3N	- ammonia - nitrogen determination by
	HACH Chemical Co. method
ALK .	<ul><li>alkalinity as CaCO<sub>3</sub> (mg/l)</li></ul>
APPROX	- approximately
MLSS	<ul><li>mixed liquor suspended solids (mg/l)</li></ul>
VSS	- mixed liquor volatile suspended
	solids (mg/l)
WASTE CONC	<ul><li>concentration of solids wasted (mg/l)</li></ul>
IGAL WASTE	- imperial gallons of solids wasted
30 MIN SET	- settled volume of 1000 ml of mixed
	liquor after 30 min. (ml)

DO	- dissolved oxygen (mg/l)
OUR	- mixed liquor oxygen uptake rate
	(mg 0 <sub>2</sub> /g-hr)
REACTOR	- aeration tank of reactor system
	referred to;
	A - SSC system
	Al- first-stage of TSC system
	A2- second-stage of TSC system
•	Bl- first-stage of TSS system or
	FSC system
	B2- second-stage of TSS system
	B3- third-stage of FSC system
	B5- fifth-stage of FSC system
RF	- raw sewage
-0900	- time sample taken
-GAM	- grab sample taken in AM
\-GPM	~ grab sample taken in PM
∳ <b>c</b>	- composite of 24 hr. by hr. samples
-C1	<ul><li>composite of 0900, 1000, and 1100</li></ul>
	hour samples
-C2	- composite of 1200, 1300, and <b>4</b> 400
	hour samples
-C3 .	<ul><li>composite of 1500, 1600, and 1700</li></ul>
	hour samples
-C4	- composite of 1800, 1900, and 2000
·	hour samples
-C5 ·	- composite of 2100, 2200, and 2300
, .	hour samples
<b>-</b> C6 .	- composite of 2400, 0100, and 0200
,	hour samples
-C7	- composite of 0300, 0400, and 0500
	hour samples
-C8	- composite of 0600, 0700, and 0800
, *	hour samples
-CC1	- composite of samples from 0900 to 1600 hr.
,	•

-CC2	- composite of samples from 1700 to 2400 hours
-CC3	- composite of samples from 0100 to 0800 hours
EAI	- sample from aeration tank Al
EA2	- clarified effluent sample from
CAC	TSC system
EA	- clarified effluent sample from
	SSC system
EB1	- clarified effluent sample from first-
	stage of TSS system
EB2	- clarified effluent sample from second-
,	stage of TSS system
EB*	- clarified effluent from FSC system
MLA	- mixed liquor sample from aeration
	tank A
MLA1	- mixed liquor sample from aeration
	tank Al
MLA2	- mixed liquor sample from aeration
	tank A2
MLB1	- mixed liquor sample from aeration
•	tank Bl
MLB2,	- mixed liquor sample from aeration tank
	B2 .
MLB1*	- mixed liquor sample from first-stage
•	of FSC system
MLB3*	- mixed liquor sample from third-stage
	of FSC system
MLB5*	- mixed liquor sample from fifth-stage
•	of FSC system
COD	<ul><li>chemical oxygen demand (mg/l)</li></ul>
BOD	- five-day, 20 <sup>0</sup> C biochemical oxygen
	demand (mg/l)
TKN	<ul><li>total kjeldahl nitrogen (mg/l)</li></ul>

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FOC - filtered organic carbon (mg/l)

NH<sub>4</sub>N - ammonia plus ammonium-nitrogen (mg/l)

NO<sub>3</sub>N - nitrate-nitrogen (mg/l)

NO<sub>2</sub>N - nitrite-nitrogen (mg/l)

Other abbreviations and symbols appearing throughout the report:

- solids retention time (days) SRT - pseudo "steady-state" run PSS-1 number 1 - dimensionless dispersion number D/uL - velocity (L/T) u - length (L) Ŀ - dispersion coefficient  $(L^2/T)$ D - analysis of variance AVOVA - dynamic or non-steady run number 1 D-1 - hydraulic residence time HRT .